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Additional Information

1 **SPEED PREDICTION MODELS FOR TRUCKS ON HORIZONTAL CURVES OF**
2 **TWO-LANE RURAL ROADS**

3
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1 ABSTRACT

2 Road safety is closely related to geometric design consistency, which is usually assessed by
3 examining operating speed. Most of consistency models only consider passenger car speeds, even
4 though the interaction between passenger cars and heavy vehicles plays a pivotal role on road
5 safety. This is due to the few models to estimate heavy vehicle speeds.

6 This study aims to develop speed prediction models for heavy vehicles on horizontal curves
7 of two-lane rural roads. To do this, continuous speed profiles were collected by using Global
8 Positioning System (GPS) tracking devices on eleven road sections. As a result, truck speeds were
9 analyzed on 105 horizontal curves.

10 The results showed that the radius of the horizontal curve and the grade at the point of
11 curvature have a great influence on heavy vehicle speeds. To this regard, vertical alignment only
12 has a significant effect on truck speeds developed along upgrades. In addition, different trends
13 were identified for loaded and unloaded trucks.

14 Several speed models were calibrated for both loaded and unloaded trucks. As a result,
15 heavy vehicle speeds were adversely affected by grades greater than 3%. This phenomenon was
16 larger for loaded trucks than for unloaded ones.

17 Finally, the calibrated 85th and 15th percentile speed models were compared with those
18 developed previously. As a conclusion, the use of the proposed models in this study was
19 recommended on Spanish two-lane rural roads due mainly to the different characteristics of heavy
20 vehicles around the world.

21
22 *Keywords: speed model, operating speed, trucks, two-lane rural roads, geometric design*
23
24

1 INTRODUCTION

2 Road safety is one of the major concerns in our society. In fact, around 1.2 million people die and
3 50 million are injured in road crashes every year (1).

4 Road crashes are mainly caused by three concurrent factors: infrastructure, vehicle, and
5 human factors. Specifically, the interaction between the infrastructure and human factors has been
6 thoroughly studied in recent years through geometric design consistency, which can be defined as
7 how drivers' expectations and road behavior relate. In this way, a consistent road minimizes
8 unexpected events to road users while driving along it, whereas an inconsistent road might present
9 numerous surprises on drivers, leading to anomalous behavior and increasing the likelihood of
10 crash occurrence.

11 The most commonly used methods to evaluate geometric design consistency are based on
12 operating speed (V_{85}). This speed is defined as the 85th percentile of the distribution of speeds
13 selected by drivers under free-flow conditions with no environmental restrictions. This speed can
14 be estimated by means of operating speed models. Although there are a lot of models which allow
15 estimating the operating speed for passenger cars on two-lane rural roads, the existing models to
16 predict the operating speed for heavy vehicles are very few. For this reason, most of the consistency
17 models are only based on the operating speed profile of passenger cars.

18 In addition, heavy vehicles should be better integrated into highway design guidelines.
19 Although the AASHTO Green Book (2) takes into account them for pavement design, climbing
20 lanes, superelevation rate, and speed deceleration on upgrades and downgrades, Spanish highway
21 design guidelines only consider them for climbing lanes and pavement design. However, the
22 interaction between both passenger cars and heavy vehicles plays a pivotal role on road safety,
23 mainly on two-lane rural roads with a large percentage of heavy vehicles (3). Therefore, not
24 considering heavy vehicle speeds might lead to inconsistent geometric designs.

25 Some authors have studied this phenomenon through the speed difference between
26 passenger cars and heavy vehicles. According to Harwood et al. (4), this speed difference might
27 cause inconsistencies on vehicle operation along upgrades. Likewise, Leisch and Leisch (5)
28 suggested that this speed difference should be limited to 15 km/h.

29 On the other hand, several research have been focused on the calibration of operating speed
30 models for heavy vehicles. Two types of models can be distinguished: those based on speed data
31 collected on rural roads and those based on dynamic and cinematic performance.

32 Regarding the first ones, Misaghi and Hassan (6) analyzed the influence of several
33 geometric variables of horizontal curves, such as the radius or the longitudinal grade, on heavy
34 vehicle speeds. Speed data were collected using electronic counter-classifiers on 20 horizontal
35 curves of two-lane rural roads. As a result, two speed models were proposed to estimate the 85th
36 percentile speed at midcurve with horizontal curve radius as explanatory variable. In addition,
37 statistically significant differences between passenger car speeds and heavy truck speeds were
38 identified, but not between passenger car speeds and light truck speeds.

39 Jacob and Anjaneyulu (7) studied the operating speed of different types of vehicle
40 (passenger cars, motorcycles, buses, and trucks) on 152 horizontal curves with grades between -
41 2% and +2%. As a conclusion, significant differences among the speeds and speed reductions
42 developed by the different types of vehicle were found. Hence, separate models were developed
43 to analyze the influence of geometry on the speeds developed by the different types of vehicle.
44 The results indicated that radius and length of the horizontal curve have a significant influence on
45 operating speed. The greater the radius or the length of the horizontal curve, the greater the speed
46 differences between the different types of vehicle.

47 However, most researchers concluded that while passenger car speed profiles can mainly

1 be based on horizontal alignment, heavy vehicle speed profiles depend on both horizontal and
2 vertical alignment. Therefore, the analysis of the operating speed differences between passenger
3 cars and heavy vehicles should be carried out considering three-dimensional (3D) geometric
4 effects. To this regard, Leisch and Leisch (5) developed a speed profile by combining the speed
5 profile estimated by considering only the horizontal alignment (assuming level grade) and the
6 speed profile only considering the vertical alignment (assuming no restrictions on the horizontal
7 alignment).

8 Likewise, several regression models were developed by Donnell et al. (3) to predict 85th
9 percentile speed of heavy vehicles on horizontal curves. These models were calibrated using a
10 combination of field data collected on 11 horizontal curves of two-lane rural roads and simulated
11 data by means of TWOPAS. In this way, it was found that an increase of the horizontal curve
12 radius and the approach tangent length was associated with greater operating speeds at the point
13 of tangency. In addition, a high grade of the approach tangent was associated with a lower
14 operating speed at this location.

15 Another study based on the horizontal and vertical alignments was developed by Gibreel
16 et al. (8). In this case, speed data were collected for all types of vehicle under free-flow conditions
17 on 9 horizontal curves combined with a sag curve and 10 horizontal curves combined with a crest
18 curve. The results showed that several parameters significantly affect the operating speed: radius
19 and deflection angle of the horizontal curve, horizontal distance between the point of horizontal
20 intersection and the point of vertical intersection, length of the vertical curve, grade, and
21 superelevation rate.

22 On the other hand, Saifizul et al. (9) studied the influence of the size and weight of heavy
23 vehicles on their speeds. To this regard, important speed differences were found when vehicles
24 were significantly different in size. In addition, when vehicles were almost similar in size and only
25 differed in the number of axles, the Gross Vehicle Weight (GVW) was the most influential factor.
26 If GVW was lower than 20 t, the speed decreased as the weight decreased. By contrast, the speed
27 remained without variation when GVW was greater than 20 t.

28 Other heavy vehicle speed profile models have been developed on upgrades based on
29 dynamic and cinematic performance (10-13). These models were mainly used for the study of
30 climbing lanes for heavy vehicles. These speed models depend on the grade, vehicle weight-to-
31 power ratio (WPR), resistance of the air, pavement characteristics, rolling resistance coefficients,
32 and friction coefficient.

33 Summarizing, few operating speed models for heavy vehicles have been calibrated. Most
34 of these studies used speed spot data collected on a low number of horizontal curves for calibration
35 and presented a large variation in model form, explanatory variables, and regression coefficients.
36 This might be due to differences in driver behavior, mechanical characteristics of the vehicles, and
37 road geometry. Thus, a single model is not universally accepted.

38 The present research analyzes truck speeds on horizontal curves of two-lane rural roads
39 based on continuous speed profiles collected using Global Positioning System (GPS) tracking
40 devices. The main advantage of this method is the large amount of continuous speed data that can
41 be collected without significant influence on drivers.

42 43 **OBJECTIVE AND HYPOTHESES**

44 The objective of this study is to analyze the influence of road geometric design on heavy
45 vehicle speeds and develop speed prediction models on horizontal curves of two-lane rural roads.
46 This research does not only focus on the evaluation of the operating speed, but also analyzes 15th

1 percentile speed, since the low percentiles encourage the emergence of traffic conflicts between
2 passenger cars and heavy vehicles.

3 The research is based on two hypotheses. The first one is that road grade, which has not
4 influence on passenger car speeds, produces lower heavy vehicle speeds on upgrades. The second
5 hypothesis is that the speed difference between heavy vehicles with similar weight is very low.
6 Despite passenger car speeds are mainly influenced by the driver, heavy vehicle speeds are
7 significantly influenced by the heavy vehicle engine performance or its weight-to-power ratio
8 (WPR).

9

10 **METHODOLOGY**

11 This research was based on continuous speed data collected through Global Positioning System
12 (GPS) tracking devices. First, GPS devices were lent to three transport companies, which equipped
13 their heavy vehicles with these devices. Next, the transport companies gave back the devices and
14 the speed data were filtered and processed. In addition, information about the use of each GPS
15 (day, time, and type of heavy vehicle) was also reported by the transport companies. The geometry
16 of the two-lane rural roads was recreated by means of the methodology proposed by Camacho-
17 Torregrosa et al. (14). Finally, different regressions models were calibrated based on the geometric
18 characteristics of the horizontal curves to estimate heavy vehicle speeds.

19

20 **DATA COLLECTION**

21 Speed data were collected between May 2015 and July 2015 in working days under dry weather
22 conditions. Heavy vehicles of three transport companies were equipped with 1Hz pocket-sized
23 Global Positioning System (GPS) tracking devices. The accuracy of these GPS devices was 2.5 m.
24 However, this accuracy is mainly composed by a general bias (common to all measurements) and
25 a minimum random error. Actually, the bias can be addressed through moving all trajectories (it
26 does not affect accuracy), while the second error is minimum (millimeters), thus being very easy
27 to address.

28 Heavy vehicles performed round trips, loaded in one direction and unloaded in the other
29 direction. All heavy vehicles were 5 axles, single trailer. The weight-to-power ratio (WPR) ranged
30 between 35 and 54 kg/kW for unloaded trucks, being its average value 43 kg/kW. However, the
31 average WPR increased to 120 kg/kW for loaded trucks.

32 Eleven two-lane rural road sections were identified from the path of the heavy vehicles
33 equipped with GPS (Table 1). All road sections are located in the Valencian Region (Spain). The
34 lane width ranged from 3.0 to 3.5 m, while the shoulder width varied from 1.0 to 1.5 m. The
35 horizontal alignment of these two-lane rural road sections was recreated using an algorithm based
36 on the heading direction (14). Additionally, the vertical alignment was obtained by means of the
37 geometric road design software Civil 3D using LIDAR data provided by PNOA (National Plan for
38 Aerial Orthography) with a root mean square error of 20 cm in height (17).

39 The GPS devices stored the following data at a 1 Hz frequency: latitude, longitude, altitude,
40 heading direction, time, and date.

41 Collected data were transformed to Universal Transverse Mercator (UTM) coordinates and
42 filtered and processed to obtain individual continuous speed profiles (15). After that, free-flow
43 conditions were checked (16). The used test is based on the hypothesis that every single driver
44 behaves in a particular way, approaching their individual speed profile to a certain speed percentile.
45 Therefore, for each individual speed profile, non-free-flow road sections were associated with
46 sudden changes in its speed profile comparing to its usual speed percentile profile.

1 After removing non-free-flow sections for every single driver, 85th and 15th percentile
 2 speed profiles were obtained. From these profiles, the minimum speed and the speed at midcurve
 3 were identified for each horizontal curve.

4 The selection of horizontal curves for speed model calibration was based on the analysis
 5 of the speed profiles. Only horizontal curves that presented a significant speed reduction were
 6 selected, i.e., those that acted as a geometric control on drivers (Figure 1). This is only possible
 7 thanks to the use of continuous speed profiles. As a result, 105 horizontal curves were considered
 8 in this research.

9 Several geometric variables of the selected horizontal curves were studied. Regarding the
 10 horizontal alignment, the most relevant variables are the radius, the length and the deflection angle
 11 of the curve, the length of the preceding tangent, and the Curvature Change Rate (*CCR*), calculated
 12 as follows:

$$13 \quad CCR = \frac{\gamma}{L} \quad (1)$$

14 where *CCR* = curvature change rate (gon/km); γ = deflection angle of the curve (gon); and *L* =
 15 curve length (km).

16 Regarding the vertical alignment, a preliminary analysis was performed to decide which
 17 variables should be included. For this, the grade at different points of the horizontal curve (at the
 18 point of curvature, at midpoint, and at the end) and the average grade were studied (Figure 2). The
 19 grade at the point of curvature corresponded to the grade along the preceding tangent. Only a few
 20 tangents presented different grades along them. In this case, the grade at the final part of the tangent
 21 was considered.

22 It was observed that the grade at the point of curvature and the grade at midpoint could
 23 better explain the phenomenon (Figure 2a and 2b). As expected, truck speeds decreased from a
 24 certain value of the grade. However, most of the selected horizontal curves presented a deceleration
 25 process that finished between these points, i.e., heavy vehicles adapted their speed before arriving
 26 at midpoint (Figure 1). Therefore, heavy vehicle speeds were mainly influenced by the grade at
 27 the point of curvature, which was similar to the grade of the preceding tangent.

28 The trend associated with the grade at the end of the horizontal curve was contrary to the
 29 intuition, i.e., the speeds got greater as grade increases (Figures 2c – unloaded trucks).
 30 Additionally, using the average grade along the horizontal curve might hide the true influence of
 31 the grade (Figure 2d).

32 So, the grade at the point of curvature was considered in this research.

33 Table 2 shows a statistical summary of the geometric variables of the selected horizontal
 34 curves.

35 The number of drivers required for each horizontal curve was also analyzed. For this, the
 36 following expression was used:

$$37 \quad n = \frac{Z^2 \cdot \sigma^2}{e^2} \quad (2)$$

38 where *n* = number of drivers required; *Z* = quantile of a normal distribution considering a 95%
 39 confidence level (1.96); σ = speed deviation (km/h); and *e* = speed error (2 km/h). As a result, the
 40 number of drivers required in most of the selected horizontal curves was lower than 10 drivers
 41 mainly due to the low speed deviation observed on these locations. To this regard, the average
 42 speed deviation was 1.96 km/h. Even though, the average number of observations was around 80
 43 drivers per horizontal curve.

44
 45
 46

1 ANALYSIS

2 Most of the previous models to predict the operating speed on horizontal curves were calibrated
3 from spot speed data collected at midcurve, assuming the minimum speed was achieved at this
4 location. However, these studies did not verify this hypothesis.

5 In this research, the minimum speed and the speed at midcurve were obtained from the
6 continuous speed profiles collected. A hypothesis test based on the analysis of paired data was
7 performed to determine whether the operating speed at midcurve (V_{85mc}) could be considered
8 similar to the minimum operating speed (V_{85min}) or not. For each horizontal curve, the following
9 hypotheses were formulated: (a) Null hypothesis $H_0: V_{85mc} - V_{85min} = 0$; (b) Alternative hypothesis
10 $H_1: V_{85mc} - V_{85min} \neq 0$. The confidence level considered in the analysis was 95%.

11 As a result, the null hypothesis was rejected ($t=0,82536$; $P\text{-Value}=0$), i.e., V_{85mc} could not
12 be considered similar to V_{85min} at a 95% confidence level. The same test was developed for the 15th
13 percentile speed obtaining the same results.

14 Therefore, the hypothesis assumed by previous research should not be accepted without a
15 preliminary analysis. In this way, the minimum speed was used for the calibration of speed
16 prediction models.

17

18 **85th Percentile Speed Model**

19 *Descriptive analysis*

20 The minimum operating speed (V_{85}) was identified for each horizontal curve from the 85th
21 percentile speed profiles.

22 A preliminary correlation analysis was carried out to identify which horizontal geometric
23 variables presented a greater influence on operating speed. This analysis found that the length of
24 the preceding tangent and the length of the horizontal curve were weakly correlated with the
25 operating speed. However, the radius of the horizontal curve, the Curvature Change Rate (CCR)
26 and the deflection angle of the horizontal curve presented a great correlation with the operating
27 speed. Specifically, the largest positive correlation coefficient was associated with the radius
28 (0.5650), while the variables with the greatest negative correlation coefficient were associated with
29 the CCR (0.7145) and the deflection angle (0.6058). Figure 3 shows these geometric variables
30 versus the operating speed.

31 The trend of the data showed clearly two different populations, loaded trucks and unloaded
32 trucks. As expected, the operating speed for loaded trucks was lower than the operating speed for
33 unloaded trucks. So, different regression analyses were performed.

34 On the other hand, the analysis of the relationship between the vertical alignment and
35 operating speed was focused on the grade. Although the grade was not a significant factor on
36 downgrades, a decreasing trend was observed from a certain value of the grade (Figure 2).

37 To verify this phenomenon, the relationship between the horizontal and vertical alignment
38 and the operating speed was studied jointly. Figure 3e shows the relationship between the radius,
39 the grade, and the operating speed. Regarding this, the size of each point represents the grade
40 value. It can be observed how operating speed decreased as the grade increased for a given radius.

41

42 *Modeling 85th percentile speed*

43 Different regression models were calibrated based on the results described above. First, several
44 speed models were developed considering different variables related to the horizontal alignment:
45 radius, CCR , and deflection angle of the horizontal curve. Different functional forms were studied.

46 This analysis allowed identifying which horizontal geometric variable presented a greater
47 influence on operating speed. The adjusted coefficient of determination was used as goodness of

1 fit. Thus, the most accurate models for loaded and unloaded trucks were based on radius as
2 explanatory variable. These models are:

$$3 \quad V_{85L} = 73.76 - 46.45/e^{0.0072 \cdot R} \quad R_{Adj}^2 = 0.60 \quad (2)$$

$$4 \quad V_{85U} = 87.55 - 60.37/e^{0.0102 \cdot R} \quad R_{Adj}^2 = 0.74 \quad (3)$$

5 where V_{85L} = 85th percentile speed for loaded trucks (km/h); V_{85U} = 85th percentile speed for
6 unloaded trucks (km/h); R = radius of the horizontal curve (m); and R_{Adj}^2 = adjusted coefficient of
7 determination.

8 The vertical alignment was introduced in the previous models to get more accurate speed
9 models. For this, an analysis of the residuals as a function of the grade was carried out. This showed
10 a homogeneous distribution of the residuals around 0 until certain value of the grade, from which
11 a decreasing linear trend was identified. Therefore, a compound model was proposed (Table 3).

12 It should be highlighted that the adjusted coefficients of determination increased
13 significantly. This improvement was more obvious for loaded trucks, so these were more
14 influenced by the grade than those unloaded. Thus, the vertical alignment had an important
15 influence on operating speed when the grade was greater than 4.23% for loaded trucks and 3.19%
16 for unloaded trucks. On the contrary, the operating speed only depended on the horizontal
17 alignment when the grade was lower than these values.

18

19 **15th Percentile Speed Model**

20 *Descriptive analysis*

21 The calibration of the 15th percentile speed models was also based on the minimum speed along
22 each horizontal curve.

23 The influence of the horizontal and vertical alignment on 15th percentile speed was similar
24 to those observed for the operating speed (Figure 3). The most influential variables related to the
25 horizontal alignment were the radius, the *CCR*, and the deflection angle of the horizontal curve,
26 which presented a correlation coefficient with respect to the 15th percentile speed equal to 0.5092,
27 -0.6835, and -0.5734, respectively. Likewise, both loaded and unloaded truck speeds described a
28 declining trend from a certain value of the grade.

29

30 *Modelling 15th percentile speed*

31 Several regression models were developed to assess the horizontal alignment influence on 15th
32 percentile speed. Different functional forms were studied and the adjusted coefficient of
33 determination was given in all regressions as goodness of fit.

34 As a result, Equations 4 and 5 show the most accurate models, which used radius as
35 explanatory variable.

$$36 \quad V_{15L} = 61.00 - 38.93/e^{0.0080 \cdot R} \quad R_{Adj}^2 = 0.54 \quad (4)$$

$$37 \quad V_{15U} = 76.66 - 56.72/e^{0.0108 \cdot R} \quad R_{Adj}^2 = 0.69 \quad (5)$$

38 where V_{15L} = 15th percentile of the distribution of speeds for loaded trucks (km/h); V_{15U} = 15th
39 percentile of the distribution of speeds for unloaded trucks (km/h); R = radius of the horizontal
40 curve (m); and R_{Adj}^2 = adjusted coefficient of determination.

41 In addition, some speed models were calibrated considering both horizontal and vertical
42 alignment. For this, the residuals of the previous models were analyzed as a function of the grade.
43 To this regard, a decreasing linear trend was identified from a certain value of the grade. Therefore,
44 the same functional form used for the operating speed was proposed (Table 4).

45 As a result, the grade showed an important influence on speed from 3.14% and 3.06% for
46 loaded and unloaded trucks, respectively. As expected, introducing the vertical alignment in the

1 models improved their accuracy, since the adjusted coefficient of determination increased
2 significantly.

3 **DISCUSSION**

4 Most previous studies about heavy vehicle speeds presented a deficient data collection due to the
5 number of locations, the number of observations, and the data collection methodology. To this
6 regard, this research has been developed from continuous speed profiles collected along a large
7 number of horizontal curves.

8
9 These continuous speed profiles allowed analyzing the hypothesis that there is not
10 significantly difference between the speed at midcurve and the minimum speed along the
11 horizontal curve. As a result, this hypothesis was rejected and the minimum speed was found to
12 be statistically lower than the speed at midcurve at a 95% confidence level.

13 The most influential variable on truck speeds was the radius. This result was consistent
14 with the outcomes obtained in previous research (3, 7, 18). Regarding this, the influence of the
15 radius on heavy vehicle speeds was different depending on whether the vehicle was loaded or not.
16 For unloaded trucks, the radius was dominant up to values around 200 m. This means that for
17 radius lower than 200 m, a small variation of the value of the radius leads to a great speed variation.
18 Conversely, a great variation of the value of the radius was needed to observe a significant speed
19 variation when the radius was greater than 200 m. However, the influence of the radius was greater
20 for loaded trucks, since a high influence was identified up to a radius of 300 m. This is due to
21 loaded trucks present a greater height of gravity center.

22 Despite identifying the same trend according to the radius of the horizontal curve, the new
23 operating speed models estimated greater speeds than the model developed by Jacob and
24 Anjaneyulu (7). This phenomenon was likely due to the difference between the Spanish and Indian
25 two-lane rural roads, which present different posted speed limits and type of heavy vehicles.
26 Regarding this, the trucks considered in this research might have a lower weight-to-power ratio
27 (WPR) than the heavy vehicles considered by Jacob and Anjaneyulu (Figure 4).

28 It should be noted that the calibrated model estimates higher speeds for loaded trucks than
29 for unloaded trucks for very low radii ($R < 20$ m). Thus, these models should be used for radius
30 greater than 20 m.

31 On the other hand, the grade at the point of curvature also showed a great influence on
32 truck speeds. Contrary to expectations, the grade threshold was lower for unloaded trucks (3%)
33 than for loaded ones (4%) according to the 85th percentile speed models. Regarding the 15th
34 percentile speed models, heavy vehicle speeds decreased for grades greater than 3% for both
35 loaded and unloaded trucks. However, the influence of the grade was higher for loaded trucks than
36 for unloaded trucks, since the regression coefficients related to loaded trucks were greater.

37 Other authors pointed out that the heavy vehicle speeds decreased as the grade increased,
38 but not from a certain grade value (3, 18). These conclusions might be related to the geometric
39 characteristics of the horizontal curves under study. Specifically, Morris and Donnell (18) studied
40 truck speeds on multilane highways, which usually present greater radius and tangent lengths. In
41 this situation, drivers are not as influenced by the horizontal alignment as on two-lane rural roads,
42 where this alignment predominates over the vertical alignment on downgrades.

43 According to Donnell et al (3), the operating speed at midcurve was also influenced by the
44 length and grade of the departure tangent. Related to this, this study concluded that the grade at
45 the point of curvature significantly influenced on truck speeds, which was similar to the grade of
46 the preceding tangent for most of the horizontal curves. So, geometric and operating characteristics

1 of the approach tangent have an important effect on horizontal curve speed, what requires further
2 research.

3 Finally, a comparison between 85th and 15th percentile speed models was carried out
4 (Figure 5). The difference between both speed percentiles got lower as the radius decreased for
5 both loaded and unloaded trucks, i.e., sharper geometries produced lower speed deviations. In
6 addition, the difference between both speed percentiles was lower for unloaded trucks than for
7 loaded ones. As an example, this speed difference was approximately 8 km/h for unloaded trucks
8 considering high values of the radius, whereas this speed difference increased to 12 km/h for
9 loaded trucks. Therefore, loaded trucks underwent a greater speed deviation than unloaded ones
10 discounting the effects of the vertical alignment.

11 Regarding vertical alignment, a different trend was observed depending on whether the
12 vehicle was loaded or not. For loaded trucks, the difference between both speed percentiles was
13 lower as the grade increased. On the contrary, the speed difference got greater as the grade
14 increased for unloaded trucks. In this way, loaded trucks presented a lower speed deviation than
15 unloaded trucks just focusing on the vertical alignment.

16 Thus, loaded trucks were more influenced by the vertical alignment and unloaded trucks
17 were more influenced by the horizontal alignment.
18

19 **CONCLUSIONS AND FURTHER RESEARCH**

20 The development of truck speed profiles is fundamental to assess the interaction between
21 passenger cars and heavy vehicles, which is a critical factor on road safety. However, this
22 phenomenon is rarely considered due to the lack of truck speed models.

23 This study presents several speed models, which include geometric characteristics as
24 explanatory variables to predict truck speeds on horizontal curves of two-lane rural roads. These
25 models were calibrated based on the minimum speed identified from continuous speed profiles
26 collected using Global Positioning System (GPS) tracking devices.

27 The results showed that the combined effect of the horizontal and vertical alignment on
28 truck speeds. The most influential variables were the radius of the horizontal curve and the grade
29 at the point of curvature. In addition, two different trends were identified which were related to
30 loaded and unloaded trucks.

31 The influence of the vertical alignment was only observed on upgrades. In this way,
32 operating speed decreased for grade values greater than 4% and 3% for loaded and unloaded trucks,
33 respectively. In addition, the influence of the grade was greater for loaded trucks than for unloaded
34 trucks.

35 85th and 15th percentile speed models were calibrated for both loaded and unloaded trucks.
36 These models were compared with those developed previously. Although the same trends were
37 observed, the use of the proposed models are only recommended on Spanish two-lane rural roads
38 due to the different characteristics of heavy vehicles among countries.

39 The models presented in this research are part of the development of a new operating speed
40 profile for heavy vehicles. The continuous speed profiles collected in this research will be used to
41 develop speed models on tangents and analyze tangent-to-curve speed variations, and acceleration
42 and deceleration rates. This will allow completing the truck operating speed profile model. In this
43 way, the interaction between passenger cars and heavy vehicles will be able to be considered in
44 the analysis of the geometric design consistency.
45

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6

7 **AUTHORS CONTRIBUTION**

8 The authors confirm contribution to the paper as follows:

- 9 • Study conception and design: Llopis-Castelló, D., Pérez-Zuriaga, A.M., and García, A.
- 10 • Data collection: Llopis-Castelló, D., Pérez-Zuriaga, A.M., and García, A.
- 11 • Analysis and interpretation of results: Llopis-Castelló, D. and González-Hernández, B.
- 12 • Draft manuscript preparation: Llopis-Castelló, D. and González-Hernández, B.

13 All authors reviewed the results and approved the final version of the manuscript.

14

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1 LIST OF FIGURE CAPTIONS:

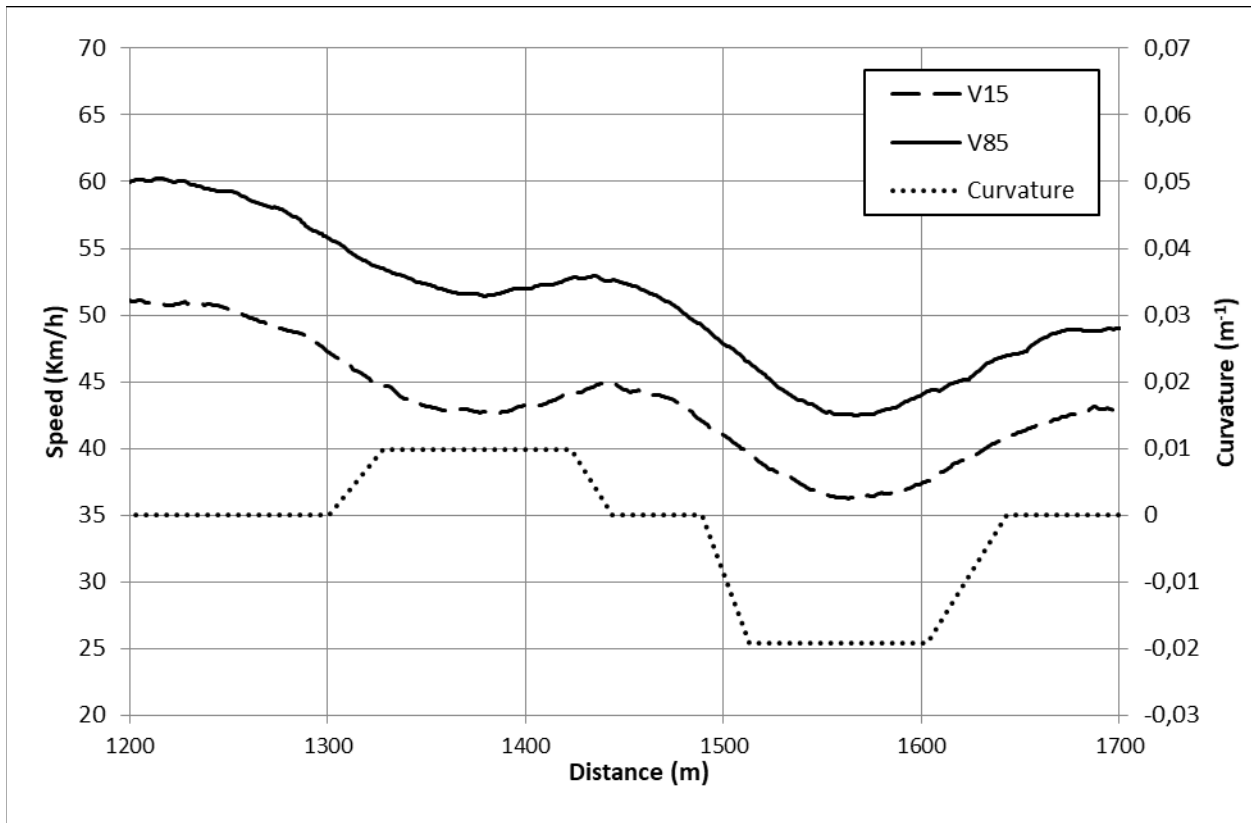
2 FIGURE 1 Horizontal curve selection.

3 FIGURE 2 Analysis of the longitudinal grade.

4 FIGURE 3 Geometric variables Vs. 85th percentile speed.

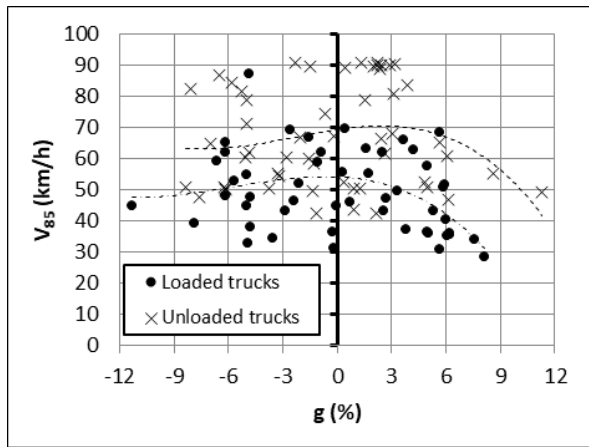
5 FIGURE 4 85th percentile speed models Vs. Jacob and Anjaneyulu's model.

6 FIGURE 5 Comparison between 85th and 15th percentile speed models.

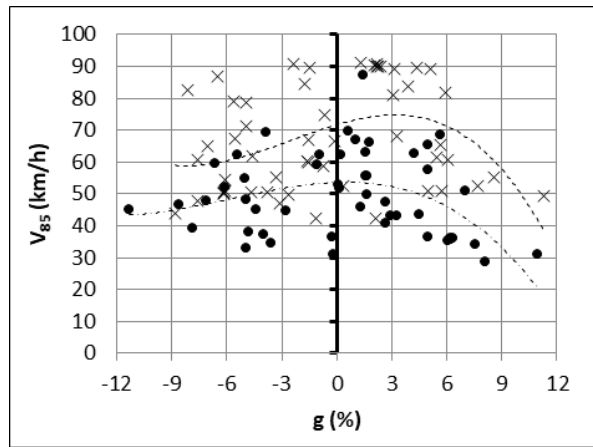


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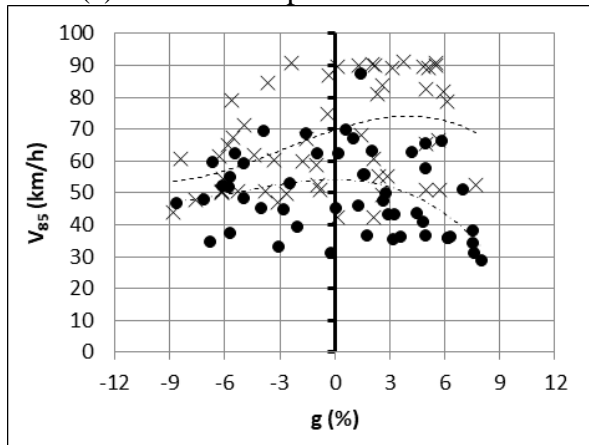
FIGURE 1 Horizontal curve selection.



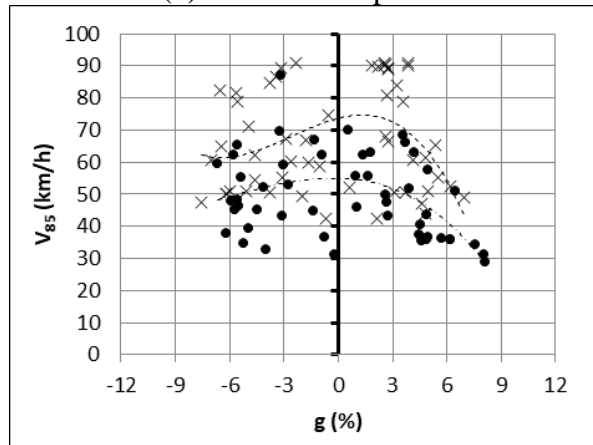
(a) Grade at the point of curvature



(b) Grade at midpoint

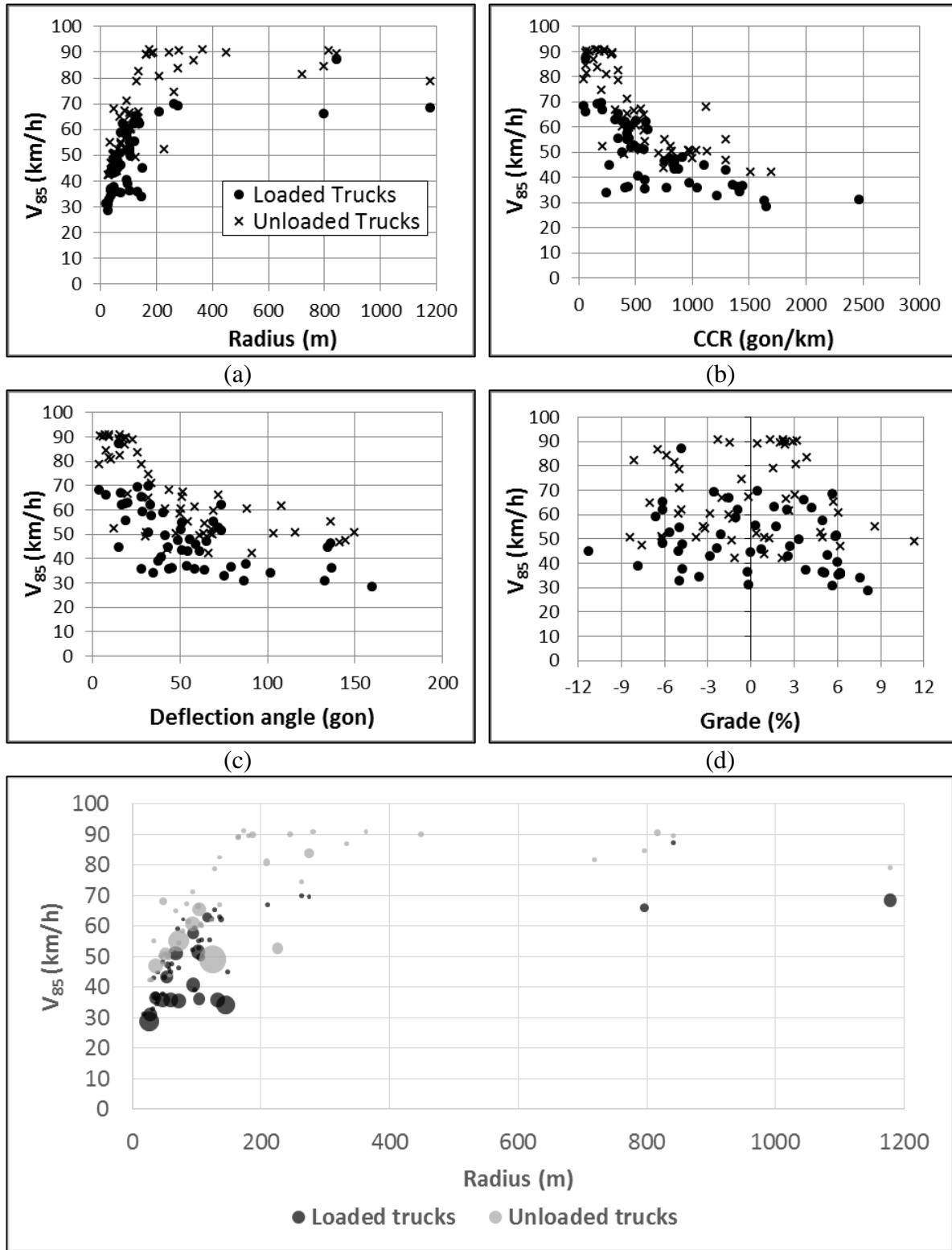


(c) Grade at the end



(d) Average grade

1
2 **FIGURE 2** Analysis of the longitudinal grade.

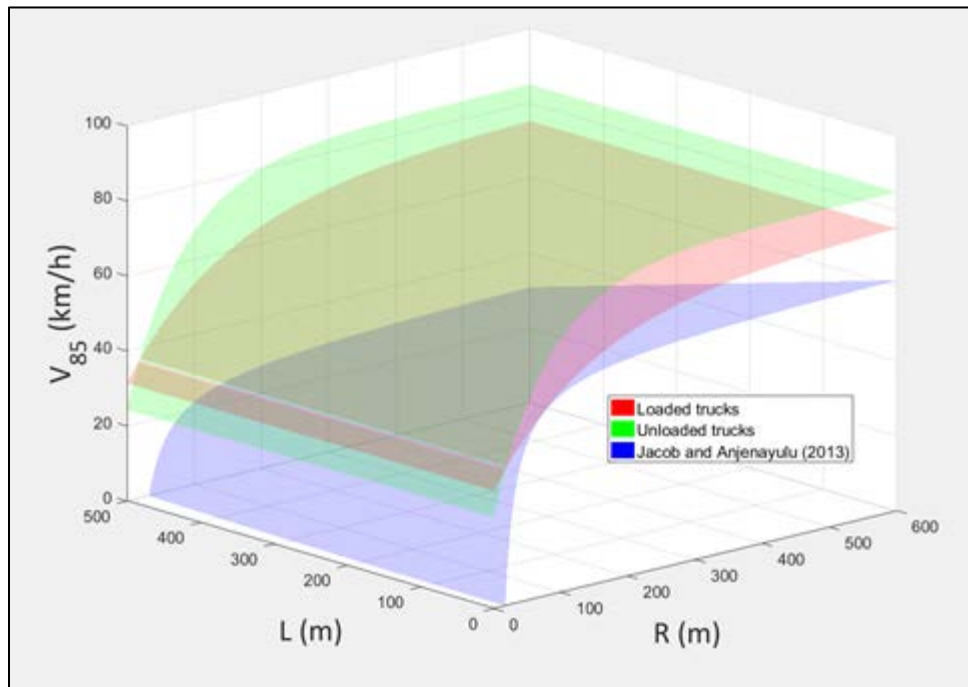


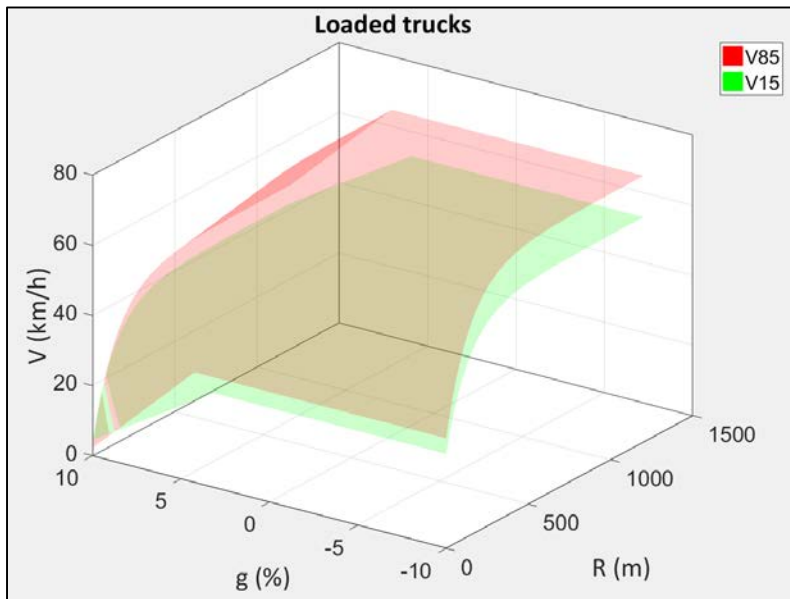
*Point size represents grade values.

(e)

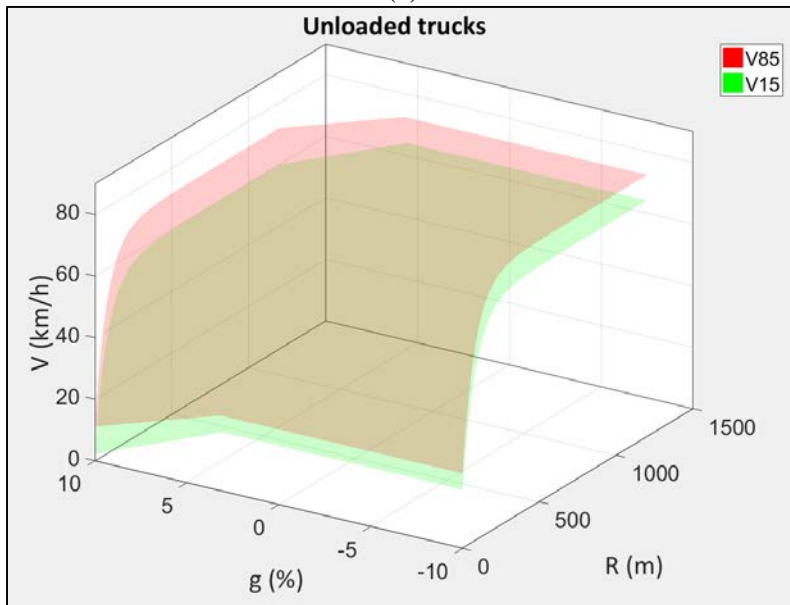
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FIGURE 3 Geometric variables Vs. 85th percentile speed.

1
2**FIGURE 4 85th percentile speed models Vs. Jacob and Anjaneyulu's model.**



(a)



(b)

1
2 **FIGURE 5 Comparison between 85th and 15th percentile speed models.**

1 LIST OF TABLES:

2 TABLE 1 Parameters of the road sections

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1 **TABLE 1 Parameters of the road sections**

Road	Segmento Name	Length (m)	Grade Range (%)
CV-425	Buñol - Alborache	1,956	[-8; +6]
CV-425	Alborache - CV-429	752	[-3; +6]
CV-425	Macastre I - Macastre II	1,419	[-5; +2]
CV-425	Macastre II - CV-580	11,996	[-9; +11]
CV-425	La Matrona I - La Matrona II	5,836	[-12; +12]
CV-345	Villar del Arzobispo - Higuieruelas	7,215	[-5; +8]
CV-600	Xávida - Fenollet	2,685	[-2; +2]
CV-610	Genovés - Cuatretonda	7,304	[-8; +10]
CV-610	Cuatretonda - Llutxent	2,686	[-3; +10]
CV-608	Llutxent - Planta	1,660	[-8; +8]
CV-610	Llutxent - CV-60	5,685	[-5; +6]

2

1 **TABLE 2 Statistical summary of geometric variables at horizontal curves**

Variable	Notation	Minimum	Maximum	Mean	Standard deviation
Radius (m)	R	18.45	1,178.36	167.43	228.52
Length of curve (m)	L	32.00	250.00	94.50	47.35
Deflection angle (gon)	γ	3.94	159.82	51.57	37.46
Length of preceding tangent (m)	L_t	7	1359	194	214
Curvature Change Rate (gon/km)	CCR	41.51	2,464.65	618.21	449.34
Grade (%)	g	-11.31	11.31	-0.04	4.68

2

1 **TABLE 3 85th percentile speed models**

Operating speed models	
Loaded trucks	$V_{85L} = \begin{cases} 75.96 - \frac{44.56}{e^{0.00685 \cdot R}} & \text{if } g \leq 4.23\% \\ 75.96 - \frac{44.56}{e^{0.00685 \cdot R}} - 5.06 \cdot (g - 4.23) & \text{if } g > 4.23\% \end{cases} \quad R_{Adj}^2 = 0.73$
Unloaded trucks	$V_{85U} = \begin{cases} 85.02 - \frac{60.62}{e^{0.01240 \cdot R}} & \text{if } g \leq 3.19\% \\ 85.02 - \frac{60.62}{e^{0.01240 \cdot R}} - 1.95 \cdot (g - 3.19) & \text{if } g > 3.19\% \end{cases} \quad R_{Adj}^2 = 0.77$
Where: V_{85L} = 85 th percentile speed for loaded trucks (km/h); V_{85U} = 85 th percentile speed for unloaded trucks (km/h); R = radius of the horizontal curve (m); and g = grade (%); and R_{Adj}^2 = adjusted coefficient of determination.	

2

1 **TABLE 4 15th percentile speed models**

15th percentil speed models	
Loaded trucks	$V_{15L} = \begin{cases} 64.17 - \frac{37.24}{e^{0.00720 \cdot R}} & \text{if } g \leq 3.14\% \\ 64.17 - \frac{37.24}{e^{0.00720 \cdot R}} - 3.28 \cdot (g - 3.14) & \text{if } g > 3.14\% \end{cases} \quad R_{Adj}^2 = 0.69$
Unloaded trucks	$V_{15U} = \begin{cases} 76.74 - \frac{57.58}{e^{0.01185 \cdot R}} & \text{if } g \leq 3.06\% \\ 76.74 - \frac{57.58}{e^{0.01185 \cdot R}} - 2.43 \cdot (g - 3.06) & \text{if } g > 3.06\% \end{cases} \quad R_{Adj}^2 = 0.74$
<p>where V_{15L} = 15th percentile speed for loaded trucks (km/h); V_{15U} = 15th percentile speed for unloaded trucks (km/h); R = radius of the horizontal curve (m); and g = grade (%); and R_{Adj}^2 = adjusted coefficient of determination.</p>	

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