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Additional Information

1 **The role of the operating parameters of SBR systems on the SMP production and on**
2 **membrane fouling reduction**

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9
10 **Abstract**

11 In this work, six identical laboratory SBRs treating simulated wastewater were operated in
12 parallel studying the effect of three food-to-microorganisms ratio (F/M ratio; 0.20, 0.35 and
13 0.50 kg COD·kg MLSS⁻¹·d⁻¹), two hydraulic retention times (HRT; 24 and 16 h) and two
14 values of number of cycles per day (3 and 6). Influence of these operational parameters on
15 the SMPs production and reactor performance, were studied. Results indicated that the
16 highest F/M ratio, HRT and cycles/day produced 72.7% more of SMP. In a second
17 experimental series, biological process yielding the maximal and the minimal SMPs
18 production were replicated and both mixed liquors (ML) and treated effluents were
19 ultrafiltrated. The flux decay in the conditions of minimum and maximum SMPs production
20 were 52% and 72%, when the SBRs effluents were ultrafiltrated while no significant
21 differences in the ultrafiltration of ML were found. In terms of permeability recovery, this
22 was lower for the case of the ML (73% and 49% of initial permeability recovered for effluent
23 and ML ultrafiltration, respectively).

24 **Keywords:** SBR; operating conditions; SMP; ultrafiltration; membrane fouling

25

26 **1. INTRODUCTION**

27 The ultrafiltration (UF) membrane technology is used as a tertiary treatment in order to
28 produce high effluent quality. This technique is used to treat the secondary effluents after
29 biological treatment to reduce the total solids, biological or chemical oxygen demand (Acero
30 et al., 2010; Norton-Brandão et al., 2013; Tchobanoglous et al., 1998) and other pollutants
31 like pharmaceutical substances (Garcia-Ivars et al., 2017; Secondes et al., 2014). On the other
32 side, UF is also used in membrane bioreactors (MBR) in order to separate the treated
33 wastewater from the mixed liquor. These processes produce water that can be reuse in the
34 agriculture or for other purposes like urban and industrial uses, aquifer recharge, etc.
35 contributing to environmental sustainability.

36 However, one of the disadvantages of the UF that avoid a wider implementation in the
37 wastewater treatment plants (WWTP) is membrane fouling. The main fouling mechanisms in
38 the UF membranes are the pore blocking (due to small colloids deposition) and cake layer
39 formation (due to build-up of particles on the membrane surface). Additionally, solutes
40 adsorption onto the membrane increases the fouling process (Boerlage et al., 2002; Mousa
41 and Al-Hitmi, 2007). On the other hand, in recent years it has been reported that the filtration
42 resistance caused by chemical potential mechanism is the cause of the primary fouling of the
43 membrane. This fouling mechanism, based on the Flory-Huggins theory, was proposed by
44 Chen et al., 2016. It was also confirmed using alginate solution to mimic the polysaccharides
45 of the extracellular polymeric substances in MBRs (Zhang et al., 2018). These authors also
46 highlighted the important role of the calcium ions on the membrane fouling.

47 In the secondary effluent the main foulant substances are the soluble organic matter. Many
48 researchers report that SMPs are the predominant components of the soluble organic matter
49 (Gkotsis et al., 2015; Schiener et al., 1994). The main SMPs components are carbohydrates,
50 proteins and humic substances (Barker and Stuckey, 1999). They are generated by three
51 mechanisms: biomass growth, substrate metabolism and biomass decay and cell lysis
52 (Laspidou and Rittmann, 2002). Thus, the mechanisms of microbial survival, under different
53 substrates or operational conditions, influences on the SMPs amounts generated during the
54 biological treatment (Wang and Zhang, 2010).

55 Concerning to the MBRs, there are more substances than in the UF process of secondary
56 effluents that contribute to the membrane fouling like sludge fractions as suspended solids,
57 colloids and dissolved solutes (Defrance et al., 2000; Fan et al., 2006) including the
58 extracellular polymeric substances (EPS), which can be accumulated on the cellular walls of
59 the microorganisms or dissolved in the reactor as SMPs (Hodgson et al., 1993; Jefferson et
60 al., 2004).

61 The role of SMPs in membrane fouling is unclear. There are researchers that reported a
62 positive correlation between SMPs productions and membrane fouling (Lee et al., 2004;
63 Rosenberger et al., 2006), while others did not observe this relationship (Drews et al., 2008).
64 On the other hand, there are not consensuses into researchers community about optimal
65 operational conditions like F/M ratio (Ghangrekar et al., 2005; Prashanth et al., 2006).

66 In this work six SBRs worked under different operational conditions. Three different F/M
67 ratio (0.20, 0.35 and 0.50 kg DQO·kg SSLM⁻¹·d⁻¹), two HRT (24 h and 16 h) and two
68 operational cycles per day (3 and 6 cycles/day) were tested. All of these values are typical in
69 SBRs operation. The first objective was to study the relationship between these conditions
70 and the biological reactors performance and their SMPs productions. This information

71 allowed obtaining the operational conditions that minimized and maximized the SMPs
72 productions. The second objective of this work was the study of the UF membrane fouling
73 working under the extreme operational conditions obtained in the first experimental step. In
74 this way, it was evaluated the membrane fouling due to SMPs and due to sludge flocs. For
75 this purpose, it was assessed by filtrating both ML (operation similar a MBR system) and
76 effluent SBR (simulating a tertiary treatment of secondary effluent).

77

78 **2. MATERIALS AND METHODS**

79 **2.1. Biological reactors**

80 *2.1.1. First experimental step: relationship between operational conditions, SBRs* 81 *performance and SMPs production*

82 In this part, the objective was to assess the relationship between the SMPs concentrations
83 produced during the municipal wastewater biological treatment and the operational
84 conditions of the SBRs. For this purpose six identical SBRs were operated with synthetic
85 wastewater (SWW), which simulates municipal wastewater, under operational conditions
86 reported in Table 1. SBRs start-up was performed with sludge taken from a MWWTP located
87 in Valencia (Spain).

88 The main components of each reactor consisted of a mechanical stirrer, two peristaltic pumps
89 and a compressor that supplied air into the SBR through two air diffusers located on the
90 reactor bottom. The system “On and Off” used in these equipments consisted of time
91 programmers connected to the electrical network. Characteristics of each cycle are presented
92 in Table 1.

Table 1. Operational SBRs conditions.

Reactor	Operating parameters			
	F/M (kg COD·kg MLSS ⁻¹ ·d ⁻¹)	HRT (h)	Cycles/day	V _{feed/draw} (L)
SBR-1	0.20			
SBR-2	0.35	24	3	2
SBR-3	0.50			
SBR-4	0.20	24	6	1
SBR-5	0.20	16	6	1.5
SBR-6	0.20	16	3	3
Cycle characteristics				
		3 Cycles/day	6 Cycles/day	
Filling + Aerobic reaction		6 h	3 h	
Sedimentation		90 min	45 min	
Draw		25 min	13 min	
Idle		5 min	2 min	

94

95

96 The SBRs (named SBR-n, where n values were between 1 and 6) were operated during 31
 97 days. The reaction volumes of all SBRs were 6 L. As it can be shown in Table 1 different
 98 feed/draw volumes and COD concentrations of feed solution were used in order to achieve
 99 the required HRT and cycles/day in the SBRs operation. In all the SBRs a concentration of
 100 2500 mg·L⁻¹ of mixed liquor suspended solids (MLSS) was maintained. Periodically sludge
 101 withdrawals were carried out to maintain this value.

102 These configurations allowed studying the influence of F/M ratio comparing the
 103 performances and the SMPs concentrations of SBR-1, SBR-2 and SBR-3. Additionally, two
 104 different HRT and two operating cycles/day were evaluated, comparing SBR-1, SBR-4, SBR-
 105 5 and SBR-6. Finally, the operational conditions that minimized and maximized the SMPs
 106 productions were obtained.

107 **2.1.2. Second experimental step: evaluation of UF membrane fouling.**

108 In this part, the objective was to study the effect of SMPs concentration on the UF membrane
109 fouling. For this purpose effluent and mixed liquor (ML) of two different SBRs were used as
110 feeds for the UF membrane. These SBRs worked under the operational conditions obtained in
111 the first experiment, which minimized and maximized the SMPs productions. When effluent
112 and ML were UF the membrane operated like a tertiary treatment or a MBR system,
113 respectively.

114 In this way, two additional SBRs were operated during 25 days according to mentioned
115 objective. The UF experiments were carried out twice in each reactor (in the second and third
116 week, named UF1 and UF2). In order to be valid the replication tests, it was previously
117 proved that SMPs concentration was the same in both feeds. Each experiment was performed
118 in two days: in the first one effluent was collected to perform the UF experiments and in the
119 second day ML was tested. This ML was returned to SBR after the experiment to maintain
120 the efficiency of the biological treatment until to perform the second test.

121 The UF module, which allowed locating a flat sheet membrane, was a Rayflow from Orelis
122 (France). Filtration was done in cross-flow mode. UP150 P membrane from Microdyn Nadir
123 (Germany) was used to carry out the experiments. The active layer material of the membrane
124 was polyethersulfone with a molecular weight cut-off of 150 kDa. Its flow rate according to
125 data supplier is $\geq 285 \text{ L}\cdot\text{m}^{-2}\cdot\text{h}^{-1}$ (with clean water, 2 bar, 20°C and cross-flow operation). The
126 effective area was 100 cm².

127 In all the experiments the cross-flow velocity was 2 m·s⁻¹ (feed flow rate = 300 L·h⁻¹) and
128 temperature was 25°C. The steps followed in each experiment were: membrane compaction at
129 transmembrane pressure (TMP) of 3 bars during 2 h, initial membrane permeability (with
130 deionised water and three TMP; 1, 2 and 3 bar), membrane fouling (with secondary effluent

131 or ML and TMP = 1 bar), membrane rinsing (30 minutes with deionised water without
132 applying TMP) and final permeability under the same conditions as the initial one. During the
133 membrane fouling, the retentate and the permeate streams were recycled to the feed tank to
134 work at constant concentration and membrane flux was measured periodically. All the
135 fouling tests were performed until stationary permeate flux value was reached (around 105
136 min).

137 **2.2. Synthetic wastewater**

138 A synthetic wastewater (SWW) with peptone, meat extract and K_2HPO_4 (supplied by
139 Panreac) diluted in tap water (mimicking municipal wastewater) was prepared for feeding the
140 SBRs. Peptone and meat extract concentrations (in equal amount) were calculated to achieve
141 the COD (Eq.(1)) to maintain the required F/M ratio .

Eq.(1)

142 where $V_R = 6$ L, $MLSS = 2500$ mg·L⁻¹, F/M was the value in Table 1 specified for each
143 reactor and $V_{feed|draw}$ was calculated according HRT and cycles/day also specified in Table 1.

144 K_2HPO_4 concentration was calculated in each case to have a relationship between COD and
145 phosphorous (COD:P) of 100:1. Table 2 shows the four different compositions of synthetic
146 wastewaters used.

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150

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Table 2. SWW preparation for the different operational conditions

Synthetic wastewater	F/M (kg COD·kg MLSS ⁻¹ ·d ⁻¹)	HRT (h)	Peptone (mg·L ⁻¹)	Meat extract (mg·L ⁻¹)	K₂HPO₄ (mg·L ⁻¹)	COD (mg·L ⁻¹)
SWW1	0.20	24	225	225	28	500
SWW2	0.20	16	149	149	18	330
SWW3	0.35	24	390	390	49	875
SWW4	0.50	24	560	560	70	1250

152

153

154 **2.3. Analysis**

155 The parameters analysed in the effluent were: pH, conductivity, turbidity, COD, total
 156 nitrogen (N_T), ammonium nitrogen (NH₄⁺-N) and total phosphorous (P_T). In the ML the
 157 suspended solids (MLSS) and volatile suspended solids (MLVSS) were measured. All of
 158 these analyses were performed twice a week.

159 Conductivity and pH were measured with an EC-Meter GLP 31+ and a pH-Meter GLP 21+
 160 both from Crison. To measure COD, N_T, NH₄⁺-N and P_T a Spectroquant NOVA 30 and
 161 reactive kits, both from Merck, were used. MLSS and MLVSS were obtained according to
 162 APHA, 2005.

163 Additionally, the sludge retention time (SRT) and the observed sludge yield (Y_{obs}) were
 164 calculated. According to the bibliography (Amanatidou et al., 2015; Klimiuk and
 165 Kulikowska, 2006), Y_{obs} allows to assess the biomass growth and it can be calculated by
 166 Eq.(2):

Eq.(2)

167 where t is the time interval between two days “i” and “j” (no sludge was withdrawn in
168 between), X_e was the mean volatile suspended solids concentrations in the effluent ($\text{mg}\cdot\text{L}^{-1}$)
169 in this time span and COD_0 and COD_e were the initial influent COD and mean COD
170 measured in the effluent at the time interval t , respectively.

171 The SMPs production was evaluated through the measurement of proteins and carbohydrates
172 concentrations in the ML. Twice a week 25 mL of ML were collected from the SBRs and
173 were centrifuged at 12000 x g. The clarified liquid was filtered at 0.45 μm . Analysis of
174 proteins was performed by BCA method (Krieg et al., 2005; Zuriaga-Agustí et al., 2013)
175 using the kits from Novagen, and carbohydrates were measured using the anthrone method
176 (Frølund et al., 1996). For it, anthrone from Panreac was used. Both methods are colorimetric
177 and the measurements of concentrations were performed with a Hach-Lange DR 5000
178 spectrophotometer. All measures were performed by triplicate.

179 **2.4. Statistical analysis**

180 An one-way ANOVA analysis (confidence level of 95 %) was carried out with Statgraphics
181 Centurion XVII in order to study the statistical significance of operational conditions in the
182 SMPs productions. The variance analyses of proteins, carbohydrates and SMPs (sum of
183 proteins and carbohydrates) concentrations have been studied. Three levels of F/M ratio
184 (0.20, 0.35 and 0.50 $\text{kg COD}\cdot\text{kg MLVSS}^{-1}\cdot\text{d}^{-1}$), two levels of HRT (24 and 16 h) and two
185 levels of cycles/day (3 and 6 cycles/day) were evaluated.

186

187 **3. RESULTS AND DISCUSSION**

188 **3.1. Relationship between operational conditions and SMPs production**

189 In the first 10 days of the SBRs operation it was considered a period of biomass acclimation
 190 to both SWW and operational conditions.

191 **3.1.1. F/M ratio**

192 The influence of F/M ratio on the biological performance was studied comparing the SBR-1,
 193 SBR-2 and SBR-3. In Table 3, the mean values with their standard deviations for some
 194 parameters of effluent and ML measured between the 11st and 31st days, are presented. The
 195 SRT corresponds to the whole experimental period.

196 **Table 3. Mean values of parameters measured for the SWW biological treatment with three different F/M**
 197 **ratios and the same HRT and cycles/day (24h and 3, respectively).**

	SBR-1	SBR-2	SBR-3
	F/M = 0.20	F/M = 0.35	F/M = 0.50
Effluent			
pH	7.4 ± 0.4	7.4 ± 0.8	7.6 ± 0.3
Conductivity (µS·cm ⁻¹)	1226 ± 57	1528 ± 155	1943 ± 107
Turbidity (NTU)	0.4 ± 0.4	2.4 ± 1.3	7.0 ± 5.5
COD (mg·L ⁻¹)	34.0 ± 13.0	61.7 ± 15.4	90.3 ± 52.2
COD removal efficiency (%)	93.0 ± 2.7	92.5 ± 2.0	91.8 ± 4.9
ML			
MLVSS/MLSS	0.94 ± 0.02	0.93 ± 0.02	0.90 ± 0.02
Y _{obs} (kg MLVSS·kg COD ⁻¹)	0.28 ± 0.04	0.34 ± 0.04	0.37 ± 0.05
SRT (day)	37.5	14.4	7.3

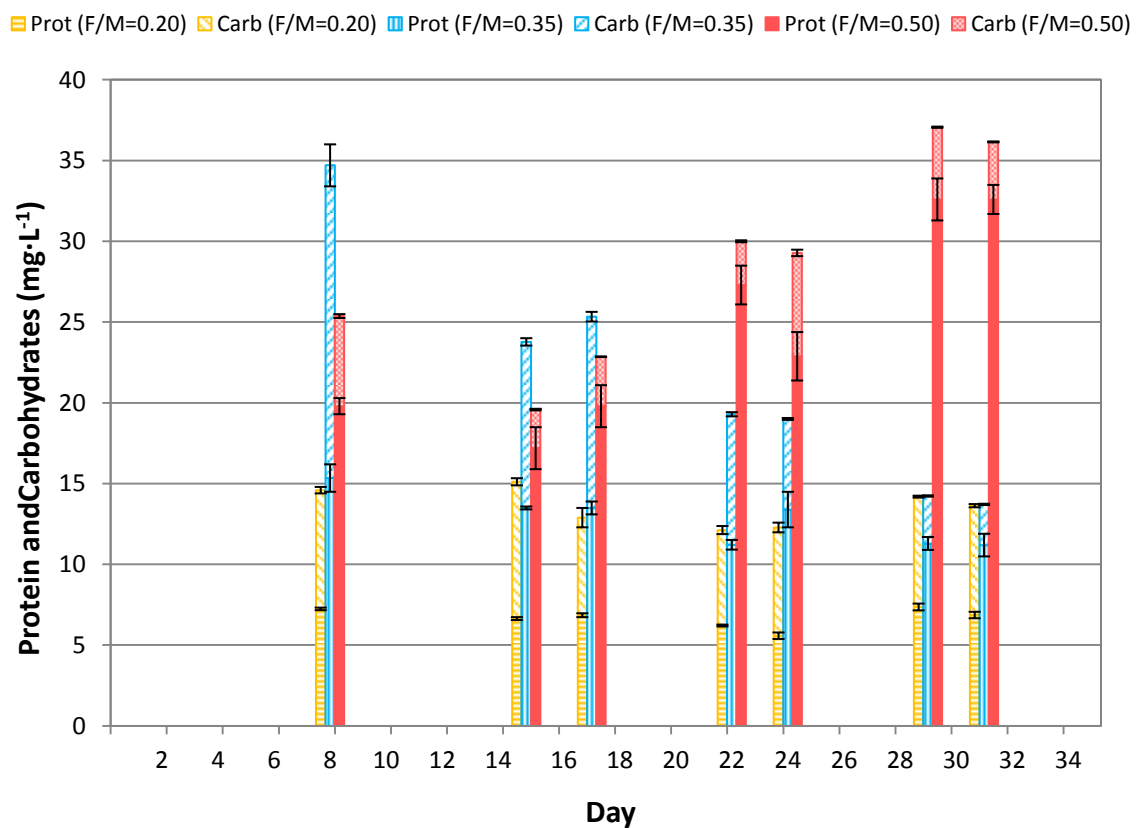
198

199

200 It can be observed that effluent pH were similar in the three SBR, meanwhile conductivity
 201 increased as F/M ratio increased, since the concentration of all the components in the SWW
 202 had to be higher in order to increase of the F/M ratio. The mean turbidity values also
 203 increased with the F/M ratio, since high F/M ratio enhanced the sludge deflocculation (Liu et
 204 al., 2012; Xie et al., 2013). In this way, fine particles remained in the supernatant.

205 Additionally, for the three F/M ratio tested in this work, the COD removal efficiency
 206 remained constant (between 91.8% and 93.0%) in the SBRs. As expected, the effluent COD
 207 and the F/M ratio varied inversely proportional. On the other hand, observing the ML values,
 208 it can be seen that Y_{obs} increased according to F/M ratio. This phenomenon is due because an
 209 increase of F/M values improve the metabolic activity and the microbial growth (Lobos et al.,
 210 2008). This fact implied more frequent sludge withdrawals to maintain the MLSS around
 211 $2500 \text{ mg}\cdot\text{L}^{-1}$. Consequently, SRT decreased when F/M ratio increased. These different SRTs
 212 affected slightly to the volatile suspended solids percentage, maintaining values between 90%
 213 and 95%.

214 The SMPs production was evaluated with the sum of proteins and carbohydrates
 215 concentrations, whose evolution for SBR-1, SBR-2 and SBR-3 are shown in Figure 1.



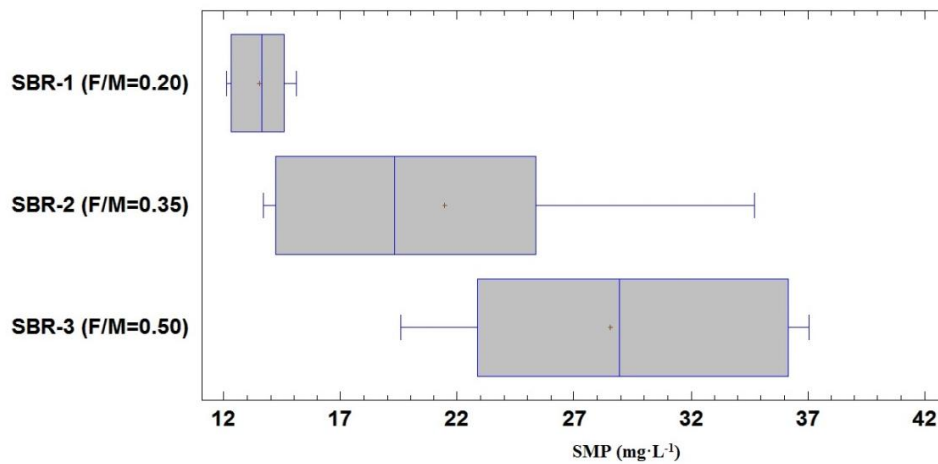
216

217 **Figure 1. Evolution of SMP concentration (protein + carbohydrates): three different F/M ratio (0.20, 0.35**
 218 **and 0.50 kg SSLM·kg DQO⁻¹·d⁻¹) and the same HRT and cycles/day (24 h and 3, respectively).**

219 Overall, it can be observed that the proportion between proteins and carbohydrates was
220 different depending on the F/M ratio. In this way, the mean proteins/SMP ratio during the
221 experimental time was $48.7 \pm 3.7\%$ and $83.9 \pm 7.1\%$ for the SBRs with lowest and higher
222 F/M ratio, respectively. For the SBR with intermediate value of F/M ratio this proportion
223 changes from the 52.7% to 77.2% at the end of experimental procedure. This phenomenon
224 can be due to the increase of cell debris in the SBRs with high F/M ratio. Since the dry
225 weight of bacterial cells of activated sludge includes 50% of proteins (Shier and Purwono,
226 1994; Xiao et al., 2017), the SBRs with more cell debris due to higher FM ratio had higher
227 proteins/SMP ratio.

228 Regarding the SMPs values, the highest concentration was also achieved in SBR-3, which
229 worked with the highest F/M ratio. This fact was confirmed by ANOVA analysis, which
230 showed a statistically significance between both parameters as expected ($F = 12.21$; $p\text{-value} =$
231 0.0004). This behavior is due because the increase of F/M ratio values provides a high
232 driving force for metabolic activity and microbial growth, so the SMPs productions increase
233 (Liu et al., 2012), as it can be shown in the Tukey diagram (Figure 2). Another parameter that
234 has an important influence on the SMPs concentration is the SRT. Some authors (Esparza-
235 Soto et al., 2011; Yu et al., 2015) reported that the SRT increase results in a decrease of the
236 generated SMP amounts, as happens in this experiment.

237 Summarizing, it can be stated that an increase of the F/M ratio resulted in a worse effluent
238 quality with higher turbidity, COD and SMP concentration.



239

240 **Figure 2. Tukey diagram for the SMP productions under three different F/M ratio (0.20, 0.35 and 0.50 kg**
 241 **SSLM·kg DQO⁻¹·d⁻¹) and the same HRT and cycles/day (24h and 3, respectively).**

242

243 3.2.2. HRT and cycles/day

244 The four SBRs whose effluents and ML characteristics are shown in Table 4 were compared
 245 to evaluate the influence of HRT and cycles/day on the SBRs performance. Like Table 3, the
 246 mean values with their standard deviations between the 11st and 31st days, were presented.

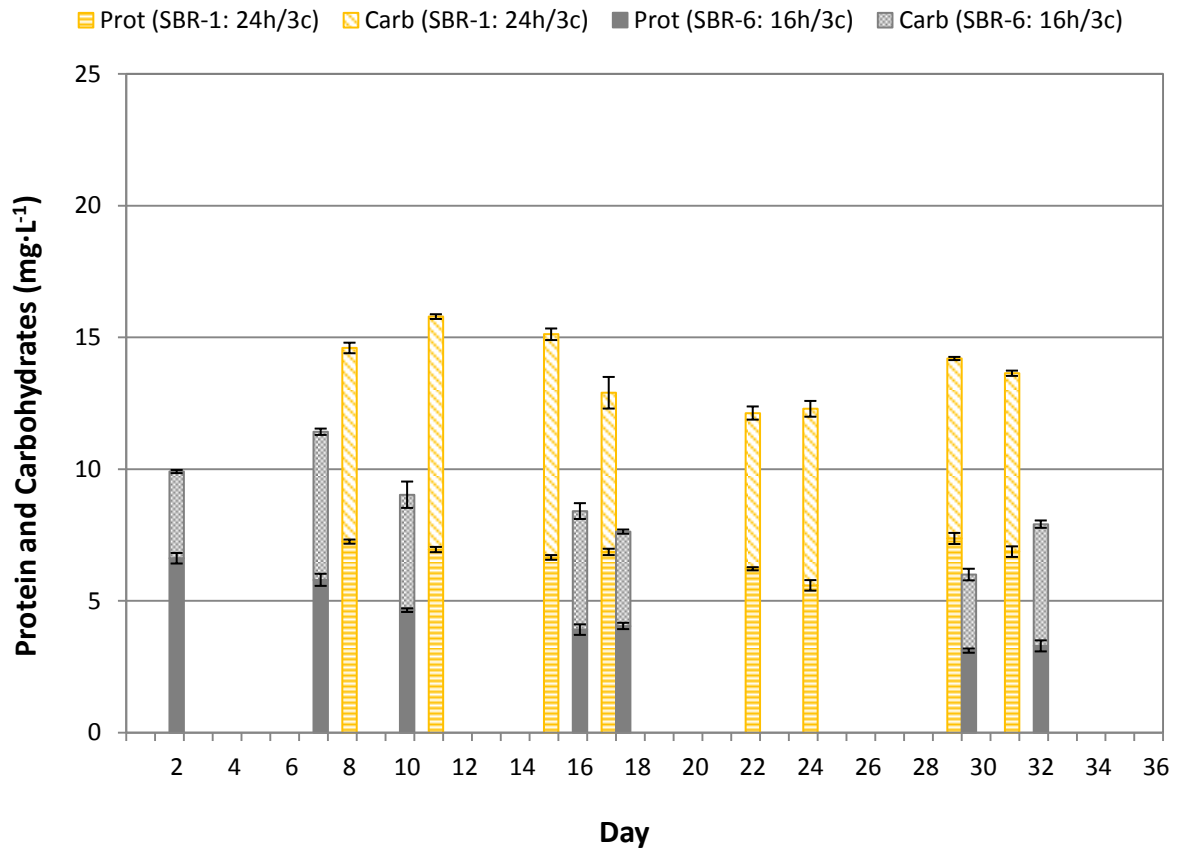
247 **Table 4. Mean values of parameters measured for the SWW biological treatment with two different HRT**
 248 **and cycles/day and the same F/M ratio (0.20 kg MLSS·kg COD⁻¹·d⁻¹).**

	SBR-1 HRT=24 h 3 cycles/day	SBR-4 HRT=24 h 6 cycles/day	SBR-5 HRT=16 h 6 cycles/day	SBR-6 HRT=16 h 3 cycles/day
Effluent				
pH	7.4 ± 0.4	7.2 ± 0.2	7.4 ± 0.4	7.4 ± 0.3
Conductivity (μS·cm ⁻¹)	1226 ± 57	1210 ± 47	1212 ± 52	1209 ± 63
Turbidity (NTU)	0.4 ± 0.4	0.3 ± 0.1	3.5 ± 6.6	0.2 ± 0.1
COD (mg·L ⁻¹)	34.0 ± 13.0	25.0 ± 7.2	33.3 ± 20.8	16.7 ± 2.5
COD removal efficiency (%)	93.0 ± 2.7	94.7 ± 1.9	87.5 ± 8.6	94.1 ± 1.1
ML				
MLVSS/MLSS	0.94 ± 0.02	0.94 ± 0.03	0.93 ± 0.02	0.93 ± 0.04
Y _{obs} (kg MLSS·kg COD ⁻¹)	0.28 ± 0.04	0.24 ± 0.07	0.34 ± 0.05	0.45 ± 0.09
SRT (day)	37.5	40.3	42.2	35.8

249 It can be observed that effluent pH, conductivity, turbidity and COD removal percentage
250 were similar in the four reactors. Therefore, it could be said that the different HRT and
251 cycles/day tested in this work, did not affect the biological treatment performance.

252 Furthermore, regarding the ML, the Y_{obs} was lower in the reactors with higher HRT because
253 endogenous respiration increased as HRT did too, diminishing the apparent biomass growth
254 (Huang et al., 2011; Luna et al., 2014). Nevertheless, the SRT was similar in the four reactors
255 as the differences in the biomass growth did not affect this parameter. No relationship was
256 observed for the two operational cycles/day values.

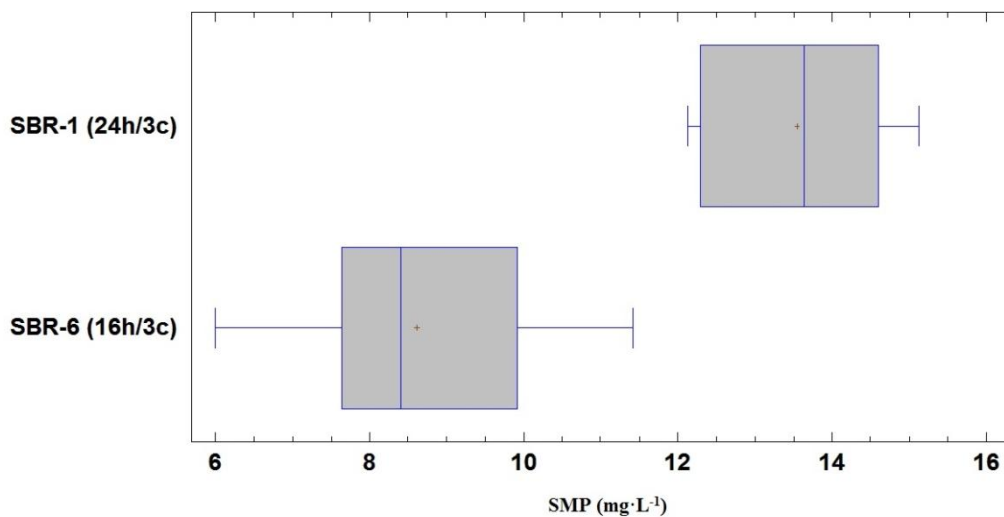
257 With respect to the SMPs production, the proteins and carbohydrates concentrations in the
258 pairs SBR-1/SBR-6 and SBR-4/SBR-5 were compared to evaluate the influence of HRT on
259 the SMP productions. In the same way, the pairs SBR-1/SBR-4 and SBR-5/SBR-6 were
260 compared to evaluate the influence of cycles/day on the SMP production. The main
261 conclusions were: proteins/SMP ratio was similar in all the reactors (around 50%) and SMPs
262 concentration was higher in the reactors operating with 6 cycles/day (10.5 ± 4.5 and $16.0 \pm$
263 $0.1 \text{ mg}\cdot\text{L}^{-1}$ for SBRs with 3 cycles and 6 cycles, respectively). In this way, the biological
264 treatment should be operated at 3 cycles/day to minimize the SMPs production. Under this
265 condition, it can be observed that the SMPs concentration was lower for the lowest HRT
266 (Figure 3 and Figure 4).



267

268 **Figure 3. Evolution of SMP concentration (protein + carbohydrates): two different HRT (24 and 16 h)**
 269 **and the same F/M and cycles/day ($0.20 \text{ kg MLSS} \cdot \text{kg COD}^{-1} \cdot \text{d}^{-1}$ and 3, respectively).**

270



271

272 **Figure 4. Tukey diagram for the SMP productions under two different HRT (24 and 16 h) and the same**
 273 **F/M and cycles/day ($0.20 \text{ kg MLSS} \cdot \text{kg COD}^{-1} \cdot \text{d}^{-1}$ and 3, respectively).**

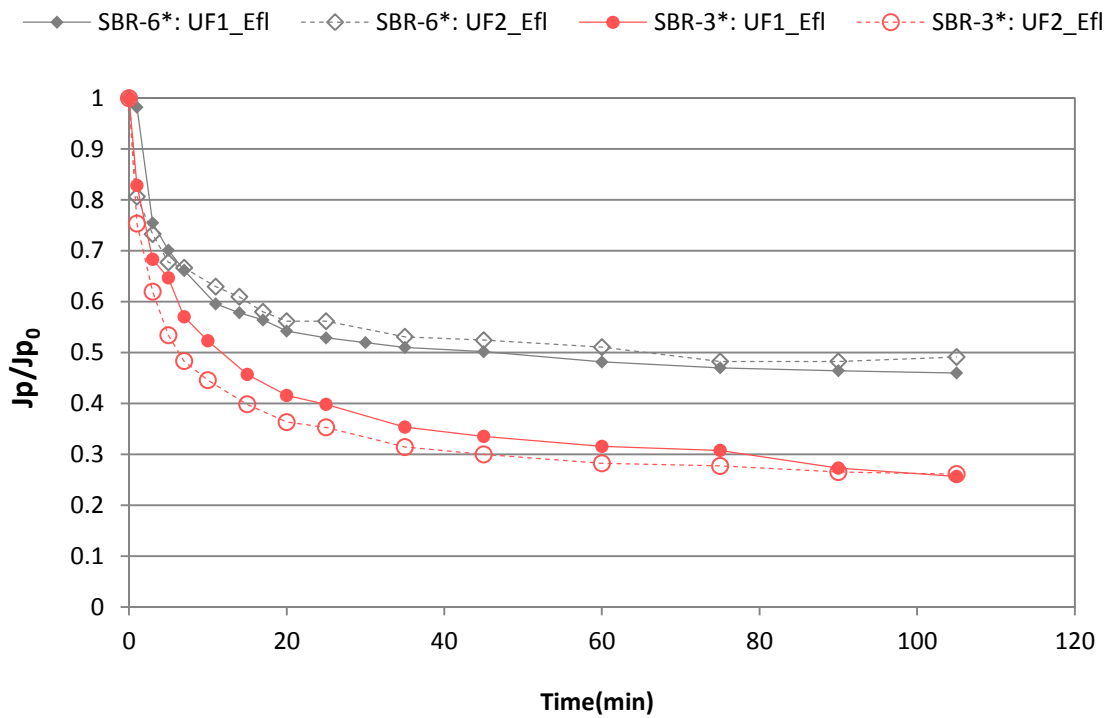
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275 This relation was confirmed by ANOVA analysis, observing a statistically significance
276 relationship between HRT and SMPs production ($F = 39.26$; $p\text{-value} < 0.0001$). This fact can
277 be explained because bacteria excrete organic materials (SMP) during starvation, which was
278 longer when HRT increased. This is due by the bacteria to obtain energy for maintenance by
279 endogenous respiration or by intracellular components metabolism (Boylen and Ensign,
280 1970; Burleigh and Dawes, 1967).

281 Finally, it can be concluded that working with F/M ratio of $0.20 \text{ kg MLVSS}\cdot\text{kg COD}^{-1}\cdot\text{d}^{-1}$,
282 HRT of 16 h and 3 cycles/day the SMPs production was minimized. Conversely, F/M ratio of
283 $0.50 \text{ kg MLVSS}\cdot\text{kg COD}^{-1}\cdot\text{d}^{-1}$, HRT of 24 h and 6 cycles/day maximized the SMPs
284 production. These conditions were performed in SBR-6 and SBR-3 with average SMPs
285 productions of 7.4 ± 1.2 and $28.6 \pm 6.5 \text{ mg}\cdot\text{L}^{-1}$, respectively. Coefficients of variation (CVs)
286 of these results were 0.17 and 0.23, respectively.

287 **3.2. Relationship between UF membrane fouling and operational conditions.**

288 As commented in methodology section, two additional reactors that minimized and
289 maximized the SMPs productions were started-up. These reactors were named SBR-6* and
290 SBR-3*, respectively. Figure 5 and Figure 6 show the UF membrane fouling experiments for
291 these SBRs, for the effluent (Efl) and mixed liquor (ML), respectively. In these figures the
292 normalized flux (J_p/J_{p0}) was plot around the experimental time. Two replicates for each
293 reactor, named UF1 and UF2, were performed. The average flow rate of the four membranes
294 used in the test were $343 \pm 19 \text{ L}\cdot\text{m}^{-2}\cdot\text{h}^{-1}$ (with clean water, 2 bar, 25°C and cross-flow
295 operation). This value is near to reported by supplier for similar conditions ($\geq 285 \text{ L}\cdot\text{m}^{-2}\cdot\text{h}^{-1}$).

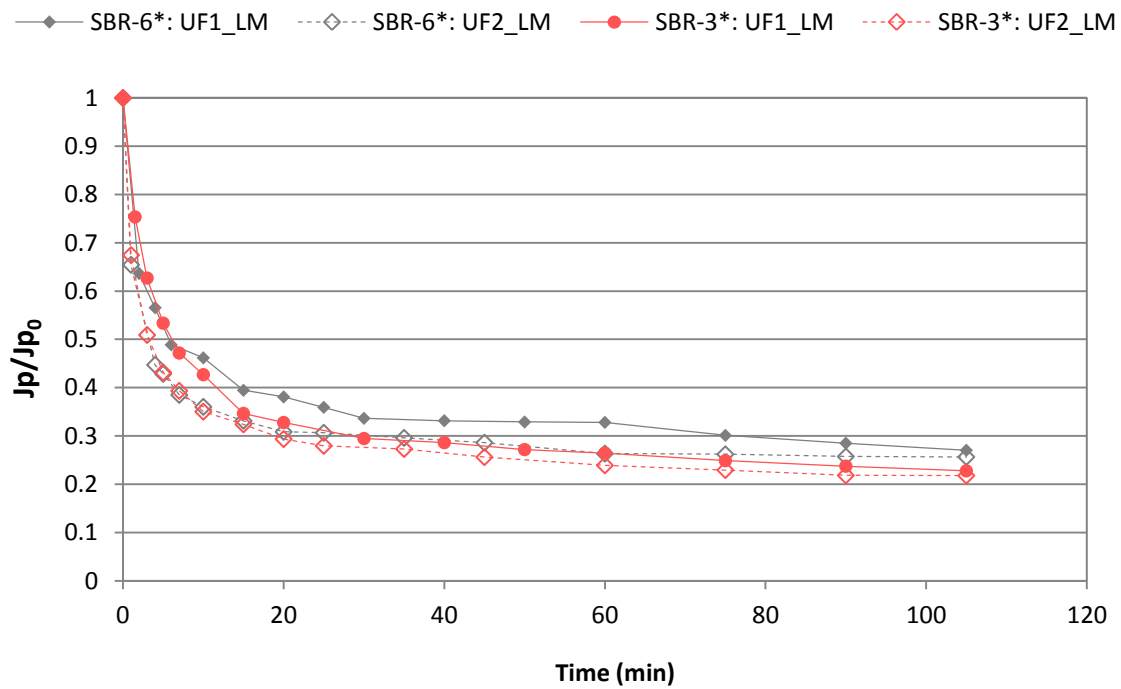


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Figure 5. UF membrane fouling experiments for the effluent (Efl) of SBR-6* and SBR-3*. Two replicates for each reactor: UF1 and UF2.

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301
302

Figure 6. UF membrane fouling experiments for the mixed liquor (ML) of SBR-6* and SBR-3*. Two replicates for each reactor: UF1 and UF2.

303 The membrane fouling was significantly higher when the effluent from SBR-3* was
304 ultrafiltered, as observed in Figure 5. The fluxes decline between initial and stationary
305 conditions (from 60 min) in both experimental days were around 72% for SBR-3* and 52%
306 for SBR-6*. The average SMPs concentrations were 14.9 ± 3.1 and 21.5 ± 5.3 mg·L⁻¹ in
307 SBR-6* and SBR-3*, respectively. CVs of these results were 0.16 and 0.24, respectively.
308 These values are very similar to those achieved in experiments with reactors SBR-6 and
309 SBR-3, which confirmed that both experiments were comparable and measurements SMPs
310 were performed with a reliable method. This is in concordance to the fact that SBR-3* was
311 the highest SMPs producer due to the high F/M ratio (0.50 kg MLVSS·kg COD⁻¹·d⁻¹) and the
312 low SRT (10 days) in this reactor. In other words, a significant difference in the SMPs
313 concentration will affect to the UF performance significantly.

314 However, when the MLs were ultrafiltered, similar flux declines were observed in both
315 reactors (around 72% and 76% in SBR-3* and SBR-6*, respectively). That can be explained
316 considering that the main substances that caused the membrane fouling in this case were the
317 sludge flocs, forming a cake layer that decreases at the same extent J_p in both cases. This fact
318 is contrary to the results reported by Fan et al., (2006), which concluded that MLSS had little
319 impact on the critical flux in a MBR operated with municipal wastewater. However, other
320 authors concluded that cake layer is the main mechanism in MBR treating municipal
321 wastewater (Wang et al., 2007; Zuthi et al., 2017). It has to be underlined that the flux
322 declines in both effluent and ML ultrafiltration from SBR-3* were very similar. It may be
323 probably due to the fact that in the secondary effluent the SMPs are transported more easily
324 into the membrane pores (what implies a severe flux decline), meanwhile the aforementioned
325 cake layer formed by the suspended solids of the ML hinder their transport to the membrane
326 pores.

327 After the rinsing step, irreversible membrane fouling was higher for the ML experiments.
328 The percentages of the initial permeability recovered were around 73% in the experiments
329 performed with effluents for both reactors. In the same way, this percentage decreased to
330 49% when membranes worked with ML.

331

332 **4. CONCLUSIONS**

333 The results of this work reveal that F/M ratio had a positive correlation with the SMPs
334 production based on the rise of the protein concentration. Additionally, the same positive
335 correlation was observed for both HRT and cycles/day parameters. In this way, the maximal
336 ($28.6 \pm 6.5 \text{ mg}\cdot\text{L}^{-1}$) and the minimal ($7.4 \pm 1.2 \text{ mg}\cdot\text{L}^{-1}$) SMPs production occurred for the
337 highest and lowest values of F/M ratio, HRT and cycles per day, respectively.

338 In the UF experiments, the flux decay was 27.8% higher when effluent from the SBR with
339 the highest SMPs concentration was UF. No significant differences were found when mixed
340 liquors were treated. On the other hand, permeability recovery after rinsing was lower in the
341 ML test (73% and 49% of initial permeability recovered for effluent and ML experiments,
342 respectively). This behavior indicates that irreversible fouling was higher when ML was
343 ultrafiltrated.

344

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348

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