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Additional Information

1 **Maximising resource recovery from wastewater grown**  
2 **microalgae and primary sludge in an anaerobic membrane co-**  
3 **digestion pilot plant coupled to a composting process.**

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11  
12 **Abstract**

13 A pilot-scale microalgae (*Chlorella* spp.) and primary sludge anaerobic co-digestion  
14 (ACoD) plant was run for one year in an anaerobic membrane bioreactor (AnMBR) at  
15 35 °C, 70 d solids retention time and 30 d hydraulic retention time, showing high  
16 stability in terms of pH and VFA concentration. The plant achieved a high degree of  
17 microalgae and primary sludge substrate degradation, resulting in a methane yield of  
18 370 mLCH<sub>4</sub>·gVS<sub>inf</sub><sup>-1</sup>. Nutrient-rich effluent streams (685 mgN·L<sup>-1</sup> and 145 mgP·L<sup>-1</sup> in  
19 digestate and 395 mgNH<sub>4</sub>-N·L<sup>-1</sup> and 37 mgPO<sub>4</sub>-P·L<sup>-1</sup> in permeate) were obtained,  
20 allowing posterior nutrient recovery. Ammonium was recovered from the permeate as  
21 ammonia sulphate through a hydrophobic polypropylene hollow fibre membrane  
22 contactor, achieving 99% nitrogen recovery efficiency. However, phosphorus  
23 recovery through processes such as struvite precipitation was not applied since only  
24 26% of the phosphate was available in the effluent. Composting process of the  
25 digestate coming from the ACoD pilot plant was assessed on laboratory-scale Dewar  
26 reactors, as was the conventional sludge compost from an industrial WWTP digestion  
27 process, obtaining similar values from both. Sanitised (free of *Escherichia coli* and

1 *Salmonella* spp.) and stable compost (respirometric index at 37 °C below  
2 0.5 mgO<sub>2</sub>·g organic matter<sup>-1</sup>·h<sup>-1</sup>) was obtained from both sludges.

### 3 **Keywords**

4 composting; anaerobic co-digestion; microalgae; resource recovery; nutrients;  
5 methane

## 6 7 **1 INTRODUCTION**

8 Classical Wastewater Treatment Facilities (WWTF) in which waste is derived from the  
9 purification process are now being replaced by new Water Resource Recovery Facilities  
10 (WRRF) in which waste is re-used to generate products of agronomic and commercial  
11 interest instead of being simply managed. Research in the 21<sup>st</sup> century is focused on  
12 wastewater management through anaerobic digestion (AD) processes, which is a  
13 promising approach combined with membrane separation and is generating increasing  
14 interest in the scientific community. Indeed, anaerobic membrane bioreactors (AnMBRs)  
15 are being applied to wastewater treatment for their several advantages, which allow higher  
16 resource recovery from wastewater at a lower cost than conventional biological aerobic  
17 systems (Becker et al., 2017; Dereli et al., 2012; Giménez et al., 2011; Robles et al., 2018).

18 Autotrophic microalgae-based technology is also being used for nutrient removal from  
19 the waterline (Acién et al., 2016; González-Camejo et al., 2020; Khalid et al., 2019).

20 Microalgae are able to hold back a higher nutrient concentration than conventional  
21 treatments and generate better clarified effluent and sludge with a higher concentration  
22 of ammonium and phosphate (Acién et al., 2016; González-Camejo et al., 2020).

23 Wastewater-grown microalgae biomass can be harvested and used as a substrate for AD.

24 Indeed, microalgae biomass AD generated in a membrane photobioreactor pilot plant  
25 (MPBR) has been reported to be efficient in terms of methane production (Greses et al.,

1 2017) on a laboratory scale. However, this efficiency could be improved by digesting the  
2 microalgae biomass with primary sludge as a co-substrate (Serna-García et al., 2020a;  
3 Solé-Bundó et al., 2019). These authors obtained better biodegradability percentage when  
4 digesting microalgae and primary sludge on a lab-scale comparing to microalgae AD as  
5 unique substrate. However, this process needs to be evaluated at pilot-scale as a first step  
6 for future industrial application.

7 In this sustainable scheme for wastewater treatment, AD also has a nutrient recovery  
8 potential. Traditional wastewater treatment plants (WWTPs) used to remove nitrogen (N)  
9 from the effluent through a biological nitrification/denitrification step and phosphorus (P)  
10 through enhanced biological P removal or chemical precipitation. However, there are  
11 other techniques that make nutrient recovery from AnMBR effluents a feasible option for  
12 a circular economy-based scenario. Although struvite precipitation is a useful alternative  
13 for recovering both P and N, at least 50 ppm of phosphate ( $\text{PO}_4\text{-P}$ ) are needed to make it  
14 profitable (Cornel et al., 2009). Modifications have thus been proposed in the WWTP  
15 layout to increase this  $\text{PO}_4\text{-P}$  concentration in AD (Martí et al., 2008). Although high P  
16 recovery efficiencies (80-90%) can be obtained through struvite precipitation, N recovery  
17 is not highly efficient (20-30%) and other technologies can be used such as  
18 bioelectrochemical systems, electrodialysis or hollow-fibre membrane contactors  
19 (HFMC). HFMC appears to be an interesting treatment because of its low volume and  
20 energy requirements. In these systems, free ammonia nitrogen (FAN) passes through a  
21 microporous hydrophobic membrane and a sulphuric acid solution is used as the draw  
22 solution to recover N as valuable ammonia sulphate.

23 As not only nutrient-rich permeate, but also nutrient-rich digestate is obtained from  
24 AnMBR processes (Nag et al., 2019, Nkoa, 2013; Seco et al., 2018), the digestate has  
25 potential agricultural applications since it could be used as a fertiliser. However, direct

1 land application of digestate presents some drawbacks: i) large agricultural areas are  
2 needed to directly apply the large amount of digestate generated in AD plants, involving  
3 high transport costs (Fuchs and Drosch, 2013); ii) in some cases, especially when digesting  
4 substrates with high slowly biodegradable volatile solids (VS) content, such as  
5 microalgae, the digestate still contains undigested VS and needs further stabilization; (iii)  
6 the possible presence of pathogens or heavy metals (Monlau et al., 2015). These  
7 drawbacks lead to the necessity of a forward stabilisation process to produce stable  
8 organic soil improver.

9 Composting has been shown to be an effective process for treating different organic  
10 wastes including anaerobic digestate, municipal solid wastes and manure wastes, among  
11 others. However, the composting process of an anaerobic digestate from a microalgae and  
12 primary sludge co-digestion plant has not yet been evaluated, to the best of the authors'  
13 knowledge. In a composting process, the biological decomposition of organic waste takes  
14 place under controlled aerobic conditions, involving mesophilic and thermophilic  
15 microorganisms. Organic substrates are transformed into a stabilised material free of  
16 pathogens and ready to be used in agriculture. This process depends not only on  
17 environmental factors such as pH, aeration, moisture content or temperature, but also on  
18 sludge characteristics such as nutrient content, particle size or carbon to nitrogen (C/N)  
19 ratio (Nikaeen et al., 2015). The C/N ratio is one of the most important parameters of the  
20 composting process (Gao et al., 2010; Puyuelo et al., 2011) since is used as an initial  
21 requirement to provide the optimum conditions for development of microorganisms and,  
22 as a monitoring parameter. Due to the low C/N ratio of anaerobic digestate, especially  
23 when treating substrates with high N content such as microalgae or sludge (Solé-Bundó

1 et al., 2019; Ullah et al., 2015), a bulking agent (BA) can be added to generate a mixture  
2 with an appropriate C/N ratio (20-25) (Huang et al., 2004).

3 There are a wide variety of microorganisms in a composting system, the most abundant  
4 being fungi, actinomycetes and bacteria (Silva and Naik, 2007). According to the Spanish  
5 Regulation RD 506/2013 (Annex IV), compost is required to contain less than 1000 most  
6 probable number (MPN) of *Escherichia coli* (*E. coli*) per gram of final product and  
7 *Salmonella* spp. has to be absent in 25 grams.

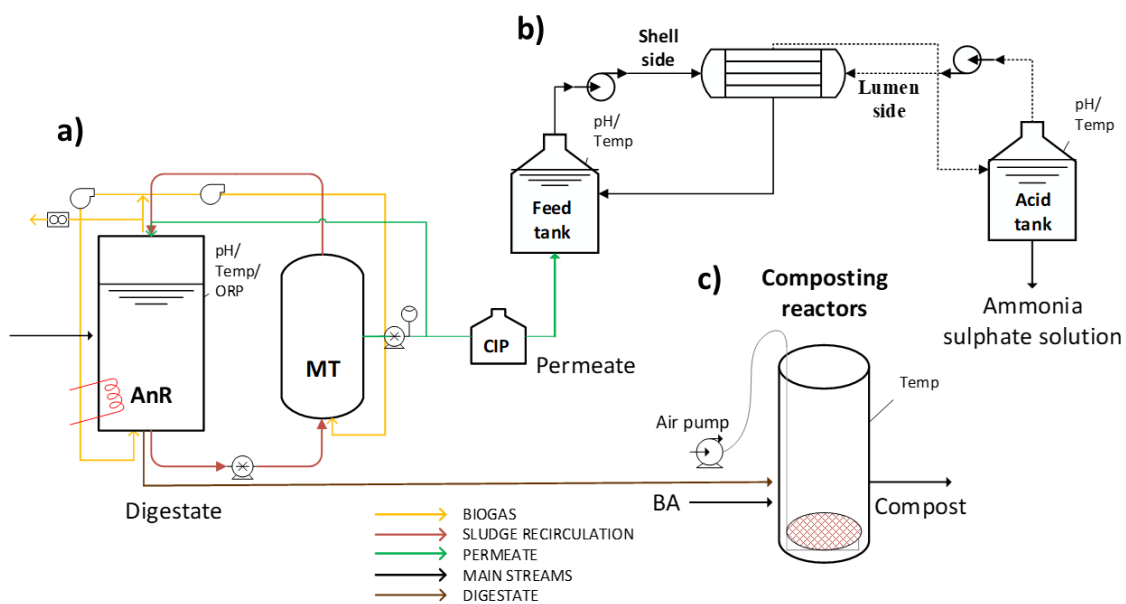
8 In this work the long-term anaerobic co-digestion (ACoD) of raw microalgae biomass  
9 and primary sludge was evaluated at pilot-scale. Potential nutrient recovery (P and N)  
10 from the ACoD permeate through struvite crystallization and HFMC was evaluated. A  
11 laboratory-scale composting process of the ACoD sludge was also applied. Composting  
12 parameters that ensured the generation of a stable organic improver were assessed. ACoD  
13 sludge composting was compared to a conventional sludge composting process to  
14 determine whether the presence of microalgae substrate in the digestate influenced the  
15 composting process in this first study to evaluate complete resource recovery from  
16 microalgae and primary sludge co-digestion.

## 17 **2 METHODS AND MATERIALS**

### 18 **2.1 Anaerobic co-digestion pilot plant description**

19 An ACoD pilot plant located in Cuenca del Carraixet WWTP (Valencia, Spain) was used  
20 for the ACoD experiments. This plant consisted of an anaerobic digester, with a total  
21 working volume of 900 L coupled to a 1-L membrane tank fitted with a 0.42 m<sup>2</sup> hollow  
22 fibre ultrafiltration membrane unit (0.03 µm pores, PURON® KMS, USA) (Figure 1a).  
23 The pilot plant was operated for one year at a solids retention time (SRT) of 70 d, while  
24 hydraulic retention time (HRT) was set at 30 d (an extensive description of the ACoD

1 pilot plant can be found in Serna-García et al., 2020b). The ACoD pilot plant feedstock  
 2 was a mixture of microalgae biomass cultivated in a MPBR plant (González-Camejo et  
 3 al., 2017) and primary sludge from the WWTP thickener. Both these substrates were fed  
 4 in proportions according to the results from previous studies (Serna-García et al., 2020a):  
 5 62% primary sludge and 38% microalgae based on VS content. The substrates were  
 6 diluted before being fed to the reactor until the desired organic loading rate (OLR) was  
 7 achieved:  $0.5 \text{ gCOD} \cdot \text{d}^{-1} \cdot \text{L}^{-1}$  and mixed in an equalisation tank.



8

9 **Figure 1:** Layout of the experimental set-up: anaerobic co-digestion pilot plant (a),  
 10 HFMC set-up (b) and an example of composting reactor (c). AnR: Anaerobic reactor,  
 11 MT: membrane tank, CIP: clean-in-place tank, BA: bulking agent.

## 12 2.2 HFMC set-up

13 A hydrophobic polypropylene HFMC (X50 2.5x8 Liqui-Cel®, USA) with a surface of  
 14  $1.4 \text{ m}^2$  was used for lab-scale N recovery. Two closed tanks of 1.2 L were used to store  
 15 the permeate and acid solution (Figure 1b). Each tank was equipped with pH and  
 16 temperature electronic sensors (SP10T, Consort®, Belgium) connected to a  
 17 multiparametric analyser (Consort® C832, Belgium). The acid stream ( $0.05 \text{ M H}_2\text{SO}_4$ )

1 circulated in the lumen side at a flow rate of  $0.4 \text{ L}\cdot\text{min}^{-1}$  while the permeate was fed to  
2 the shell side at a flow rate of  $0.6 \text{ L}\cdot\text{min}^{-1}$  and a pH of 10. Since the different ammonia  
3 ( $\text{NH}_3$ ) concentrations on each side of the membrane are the driving force, it is necessary  
4 to work at pH over 8.6 in the feeding solution so, pH was adjusted by sodium hydroxide  
5 (1M). Both streams were recirculated and fed counter-currently. Filtration and settling  
6 were applied as pre-treatment to avoid membrane clogging.

## 7 **2.3 Composting experiments**

### 8 *2.3.1 Reactors description*

9 Seven cylindrical composting reactors ( $R_{A1}$ - $R_{A4}$  and  $R_{C1}$ - $R_{C3}$ ) were operated at lab-scale.  
10 The composting reactors were 4 L glass Dewar flasks (KGW Isotherm, Germany) covered  
11 by an aluminium coating with cork insulation between the inside and the outside walls.  
12 A plastic mesh was incorporated at the bottom of all the reactors covered by a gravel layer  
13 to separate leachate and composted material. The Dewar flask covers were perforated to  
14 allow gas evacuation. A layout of the composting reactor is shown in Figure 1c.

### 15 *2.3.2 Composting substrates*

16 Two types of sludge were used to generate the mixtures in each composting reactor. The  
17 first, henceforward called 'ACoD sludge', was obtained from the ACoD pilot plant  
18 described in Section 2.1. The second, henceforward called 'conventional sludge', was an  
19 anaerobic sewage sludge from the Carraixet WWTP's conventional AD process, operated  
20 at an SRT of 20 d. Conventional sludge was used as reference substrate to compare the  
21 composting results obtained with the ACoD sludge under study. Both sewage sludges  
22 were pre-treated in a centrifuge to remove excess moisture, achieving values around 80-  
23 87% moisture. A cationic polyelectrolyte was added to conventional sludge that had been  
24 dried in the industrial centrifuge of the Carraixet WWTP. ACoD sludge was dried in a  
25 lab-scale centrifuge (Eppendorf, Centrifuge 5804) at 4350 rpm for 30 minutes and the



1 obtained pellet was centrifuged for a further 15 minutes. The ACoD sludge was left to  
2 air-dry for 48 hours before its use.

3 Five different BA were characterised (Table 1). Pruning remains from the University of  
4 Valencia's garden were chosen as BA for the analytical results and their availability.  
5 These remains were shredded to achieve the correct size for assimilation by the  
6 microorganisms involved in the process.

7 **Table 1:** Bulking agent characterisation.

<b>Bulking agent</b>	<b>C/N ratio</b>	<b>Nitrogen (%)</b>	<b>Moisture (%)</b>
Pruning remains	49.5± 0.2	0.90± 0.00	49.1± 0.4
Lawn	13.9± 0.5	3.08± 0.09	78.9± 0.8
Olive wood	109 ± 1	0.42± 0.02	48.3± 1.1
Cypress tree	58.4± 0.3	0.81± 0.01	57.9± 0.2
Orange tree	29.4± 0.9	1.50± 0.02	30.6± 0.6

8

9 **2.3.3** *Experimental design*

10 The reactors were operated for a maximum of 44 days in pairs to evaluate the effect of  
11 different operating conditions on the composting process. Three parameters were  
12 assessed: i) aeration, ii) addition of inoculum and iii) mixture of sludge and BA. Table 2  
13 shows the main operating conditions of the seven reactors. Reactors R<sub>A1</sub> to R<sub>A4</sub> were fed  
14 with ACoD sludge, while R<sub>C1</sub> to R<sub>C3</sub> were fed with conventional sludge.

15 The effect of aeration was evaluated by the pairs of reactors R<sub>A1</sub> and R<sub>A3</sub> and R<sub>C1</sub> and  
16 R<sub>C3</sub>, which were run under the same operating conditions with different aeration modes:  
17 R<sub>A1</sub> and R<sub>C1</sub> had forced aeration while R<sub>A3</sub> and R<sub>C3</sub> were turned by hand (Table 2). The  
18 effect of adding inoculum to the reactor was assessed by reactors R<sub>A2</sub> and R<sub>A4</sub>, which had  
19 the same operating conditions, but R<sub>A2</sub> had inoculum while R<sub>A4</sub> had none (Table 2). The

1 effect of mixing different proportions of sludge with BA was evaluated by the pairs R<sub>A2</sub>  
 2 and R<sub>A3</sub> and R<sub>C2</sub> and R<sub>C3</sub>, which were run under the same operating conditions but with  
 3 different proportions of sludge and BA (Table 2). The mixture component proportions  
 4 were calculated as ‘volumetric proportions’ and ‘theoretical proportions’. Volumetric  
 5 proportions were generated with 2.5 volumes of BA per sludge volume, a commonly used  
 6 ratio in sludge composting facilities. Theoretical proportions were calculated using Eq.1,  
 7 in which the weight of each mixture’s component is determined by setting the C/N ratio  
 8 at a value of 25 and the moisture content in a value between 50% and 70%.

$$9 \quad \frac{m_{BA}}{m_s} = \frac{(25 \cdot N_s(kg)) - C_s(kg)}{C_{BA}(kg) - (25 \cdot N_{BA}(kg))} \quad \text{Eq. 1}$$

10 being  $m_{BA}$  and  $m_s$ , proportions of BA and sludge, respectively;  $N_s$  and  $C_s$  the sludge N  
 11 and carbon content, respectively; and  $N_{BA}$  and  $C_{BA}$  the BA N and carbon content,  
 12 respectively.

13 **Table 2.** Main reactor operating conditions.

Reactors identification	Days in operation	Sludge	Mixture proportions	Aeration	Inoculum
R <sub>A1</sub>	36	ACoD	Theoretical	Forced	✓
R <sub>C1</sub>	36	Conventional	Theoretical	Forced	✓
R <sub>A2</sub>	44	ACoD	Volumetric	Turned by hand	✓
R <sub>C2</sub>	44	Conventional	Volumetric	Turned by hand	✓
R <sub>A3</sub>	27	ACoD	Theoretical	Turned by hand	✓
R <sub>C3</sub>	27	Conventional	Theoretical	Turned by hand	✓
R <sub>A4</sub>	31	ACoD	Volumetric	Turned by hand	-

14 *ACoD: Anaerobic co-digestion; A: ACoD sludge; C: Conventional sludge*

15 Temperature was measured daily by a temperature probe inside the reactor. R<sub>A1</sub> and R<sub>C1</sub>  
 16 were adapted to receive forced bottom-to-top aeration at a flow rate of 2 L·min<sup>-1</sup> (by air

1 supplied from the compressed air network). The rest of the reactors were manually turned  
2 over once a day. In all the reactors except R<sub>A4</sub> (Table 2) 400 mL of inoculum from the  
3 maturation stage of a compost pile from the Vintena composting plant (Carcaixent,  
4 Valencia) were added to the mixture inside the reactor to accelerate the speed reaction of  
5 the microorganisms involved in the process.

## 6 **2.4 Performance indicators**

7 The ACoD process efficiency was evaluated in terms of biodegradability percentage,  
8 biomethane potential and methane yield according to the equations previously reported  
9 in Serna-García et al. (2020a).

10 To assess nutrient recovery from the ACoD permeate, P and N recovery were calculated  
11 by Eq. 2 and Eq. 3, respectively.

$$12 \quad \% P \text{ recovery} = \frac{P_{eff}}{P_{inf} + P_{rel}} \cdot 100 \quad \text{Eq. 2}$$

13 being  $P_{eff}$  ( $\text{mgP-PO}_4 \cdot \text{L}^{-1}$ ) the phosphate concentration in the effluent,  $P_{inf}$ , ( $\text{mgP-PO}_4 \cdot \text{L}^{-1}$ )  
14 the phosphate concentration in the influent and  $P_{rel}$  ( $\text{mgP-PO}_4 \cdot \text{L}^{-1}$ ) the phosphate released  
15 into the reactor during AD. This released phosphate was calculated as the influent stream  
16 phosphate degraded during AD, according to the substrate biodegradability obtained, as  
17 will be further explained in Section 3.2.

$$18 \quad \% N \text{ recovery efficiency} = \frac{TAN_{eff,0} - TAN_{eff,end}}{TAN_{eff,0}} \cdot 100 \quad \text{Eq. 3}$$

19 being  $TAN_{eff,0}$  ( $\text{mgN-NH}_4 \cdot \text{L}^{-1}$ ) the initial concentration of Total Ammonia Nitrogen  
20 (TAN) in the HFMC influent (permeate from ACoD pilot plant) and  $TAN_{eff,end}$ , ( $\text{mgN-}$   
21  $\text{NH}_4 \cdot \text{L}^{-1}$ ) the TAN concentration at the end of the process (HFMC effluent).

22 To characterise the free ammonia transfer rate, the mass transfer coefficient was obtained  
23 using Eq. 4.

1  $\ln\left(\frac{C_0}{C_t}\right) = \frac{k \cdot A}{V} \cdot t$  Eq. 4

2 being  $C_0$  the initial total ammonia concentration in the feed solution ( $\text{g} \cdot \text{m}^{-3}$ ),  $C_t$  the total  
3 ammonia concentration in the feed solution at time  $t$  ( $\text{g} \cdot \text{m}^{-3}$ ),  $A$  the membrane surface  
4 ( $\text{m}^2$ ),  $V$  the volume of the feed solution storage tank ( $\text{m}^3$ ) and  $t$  the time (s).

5 C/N ratio, porosity and total nitrogen (TN) of both conventional and ACoD sludge  
6 mixtures with BA were measured in the initial composting samples. The presence of  
7 pathogens (*E. coli* and *Salmonella* spp.) was measured in both sludges (before being  
8 mixed with BA). Moisture and organic matter content, pH and electric conductivity were  
9 monitored weekly in the composting reactor samples to assess the process performance.  
10 The C/N ratio and pathogens were also analysed in each final product mixture.

11 According to Barrena et al. (2005), respirometric assays at the in situ temperature are  
12 suitable to monitor process biological activity since they are representative of the  
13 metabolic state of the microorganisms in the reactor. Nevertheless, assays at 37 °C are  
14 more useful to study the stability of the process. To monitor the biological activity of the  
15 composting material several static respirometric assays at process temperature were  
16 therefore performed during the composting process in the reactors that achieved  
17 thermophilic temperatures. Static respirometric assays at a fixed temperature of 37 °C  
18 were also performed in the same reactors to analyse the stability of the mixture. The slope  
19 of the oxygen concentration (%) versus time curves was calculated for each assay to  
20 calculate the respirometric index (RI). Respirometric assays and RI calculation were  
21 carried out according to Barrena et al. (2005).

## 22 **2.5 Analytical Methods**

23 Total solids (TS), VS, TSS (total suspended solids), VSS (volatile suspended solids), total  
24 chemical oxygen demand (TCOD), soluble COD, nutrients concentration (ammonium

1 (NH<sub>4</sub>-N), TN, PO<sub>4</sub>-P and total phosphorus (TP)), Alkalinity (Alk) and Volatile Fatty  
2 Acids (VFA) were measured in triplicate thrice a week according to APHA (2012)  
3 procedures. Methane content in the biogas produced was also determined thrice a week  
4 using a gas chromatograph equipped with a Flame Ionisation Detector (GC-FID, Agilent  
5 Technologies, USA). 1 mL of biogas was collected from the top of the reactor by a gas-  
6 tight syringe and injected into a 15 m × 0.53 mm × 1 μm TRACER column (Teknokroma,  
7 Spain) which was operated at 40 °C. Helium was the carrier gas at a flow-rate of 40  
8 mL·min<sup>-1</sup>. Methane pure gas (99.99%) was used as standard.

9 Moisture and organic matter content, pH and electric conductivity were measured  
10 according to Standard Methods (APHA 2012) with the corresponding dilutions for  
11 adapting the method procedure to compost samples. For instance, for electric conductivity  
12 and pH determination, the sample was previously diluted in a ratio of 1:10. The  
13 supernatant was analysed after 30 min of agitation and 20 min of centrifugation (11000  
14 rpm). C/N ratio was determined by measuring the elemental components of the mixture  
15 on an Elemental Analyser EA 1110 CHNS (CE Instruments Ltd, Wigan, United  
16 Kingdom). A previous pre-treatment of the sample, which consisted of drying the sample  
17 at 65 °C in an oven and applying a milling process, was carried out to transform the  
18 heterogeneous material into a homogenous powder. Porosity was determined by the  
19 weight difference between the original sample and the sample saturated with water. TN  
20 in composting samples was determined according to APHA (2012) with previous  
21 homogenisation of the sample in a sonicator (S250D, Branson) and subsequent dilution  
22 at a ratio 1:1000.

23 *E. coli* presence was quantitatively determined by the standard method for enumeration  
24 of *E. coli* β-glucuronidase positive, following the UNE-EN ISO 9308-1:2014. *Salmonella*  
25 spp. was measured following the UNE-EN ISO 19250 standard method.

## 1 3 RESULTS AND DISCUSSION

### 2 3.1 Anaerobic co-digestion pilot plant performance

3 Continuous pilot-scale ACoD of microalgae biomass and primary sludge was monitored  
4 over one year. Microscopic study showed that microalgae biomass consisted primarily of  
5 *Chlorella* spp. Pseudo steady state was achieved in terms of biogas production, TS and  
6 nutrient concentration after 160 days of operation and was maintained and studied for a  
7 further period of 200 days. Table 3 shows the characterised influent and mixed liquor  
8 streams from the ACoD pilot plant during pseudo steady state. The co-digestion reactor  
9 achieved a biogas production of  $78 \text{ L}\cdot\text{d}^{-1}$ , with a methane percentage around 69%. Then,  
10 a higher methane yield of  $218 \text{ mLCH}_4\cdot\text{gCOD}_{\text{inf}}^{-1}$  ( $371 \text{ mLCH}_4\cdot\text{gVS}_{\text{inf}}^{-1}$ ) was obtained  
11 than in numerous lab-scale studies of microalgae digestion as the sole substrate  
12 (González-Fernández et al., 2012; Greses et al., 2018; Ras et al., 2011, Wang et al., 2016)  
13 even when pre-treatment was applied to the microalgae biomass (Magdalena et al., 2018;  
14 Passos et al., 2014; Solé-Bundó et al., 2018, Wang et al., 2016). This methane yield  
15 corresponds to a total biodegradability percentage of 62.5% with a biomethane potential  
16 of 61.5%, which indicated that only 1% of the biodegradable organic matter was  
17 consumed by sulphate-reducing bacteria. The high AnMBR biodegradation efficiency  
18 also resulted in a high COD and VS removal of 63 and 64%, respectively. The system  
19 showed high stability since high alkalinity and no VFA accumulation was observed  
20 (Table 3), which resulted in stable non-controlled pH during the whole operation.  
21 Regarding membrane performance, pilot plant filtration was carried out at an average  $J_{20}$   
22 of 4.5 LMH and a filtration time of 180 s, showing stability. No membrane replacement  
23 was needed during the experiment, since applying a specific gas demand of  $0.15 \text{ N}\cdot\text{m}^3\cdot\text{m}^{-2}\cdot\text{h}^{-1}$ ,  
24 a backwash cycle every two filtration cycles and physical cleaning was enough to  
25 control fouling formation (Serna-García et al., 2020b).

1 **Table 3:** Anaerobic co-digestion (ACoD) pilot plant influent and mixed liquor  
 2 characterisation during pseudo steady state (mean  $\pm$  standard deviation values).

<b>ACoD pilot plant</b>	
<b>Influent</b>	
TS (mgTS·L <sup>-1</sup> )	13086 $\pm$ 2009
VS (mgVS·L <sup>-1</sup> )	9919 $\pm$ 1592
TSS (mgTSS·L <sup>-1</sup> )	11558 $\pm$ 1323
VSS (mgVSS·L <sup>-1</sup> )	8800 $\pm$ 970
COD (mgCOD·L <sup>-1</sup> )	15895 $\pm$ 1682
TN (mgN·L <sup>-1</sup> )	578 $\pm$ 113
NH <sub>4</sub> -N (mgN·L <sup>-1</sup> )	97 $\pm$ 20
TP (mgP·L <sup>-1</sup> )	137 $\pm$ 17
PO <sub>4</sub> -P (mgP·L <sup>-1</sup> )	48.0 $\pm$ 14.1
SO <sub>4</sub> -S (mgS·L <sup>-1</sup> )	194 $\pm$ 63
<b>Mixed liquor</b>	
TS (mgTS·L <sup>-1</sup> )	11337 $\pm$ 664
VS (mgVS·L <sup>-1</sup> )	7680 $\pm$ 457
TN (mgN·L <sup>-1</sup> )	685 $\pm$ 80
NH <sub>4</sub> -N (mgN·L <sup>-1</sup> )	397 $\pm$ 33
TP (mgP·L <sup>-1</sup> )	145 $\pm$ 16
PO <sub>4</sub> -P (mgP·L <sup>-1</sup> )	36.6 $\pm$ 6.1
pH	7.2 $\pm$ 0.1
VFA (mgCH <sub>3</sub> COOH·L <sup>-1</sup> )	13.1 $\pm$ 15.4
Alk (mgCaCO <sub>3</sub> ·L <sup>-1</sup> )	2058 $\pm$ 109

3 *TS: total solids; VS: volatile solids TSS: total suspended solids; VSS: volatile suspended solids;*  
 4 *COD: chemical oxygen demand; TN: total nitrogen; NH<sub>4</sub>-N: ammonium; TP: Total phosphorus;*  
 5 *PO<sub>4</sub>-P: phosphate; SO<sub>4</sub>-S: sulphate; VFA: volatile fatty acids; Alk: alkalinity.*

6 High biogas production from microalgae and primary sludge was obtained in long-term  
 7 pilot-scale operation without the need to apply costly pre-treatments to improve  
 8 microalgae degradation. This biogas was rich in methane and is a renewable fuel that  
 9 could be used for energy and heat generation allowing an approach to circular economy  
 10 scenarios in which a WRRF would be self-sufficient in terms of energy (further research  
 11 is needed).

### 1 3.2 Nutrient recovery

2 ACoD released N to the soluble phase, as expected; around  $400 \text{ mgNH}_4\text{-N}\cdot\text{L}^{-1}$  was present  
3 after the ACoD process (Table 3). Ammonium remained stable at a concentration higher  
4 than that present in the influent (Table 3). Unlike ammonium, phosphate content ( $37$   
5  $\text{mgPO}_4\text{-P}\cdot\text{L}^{-1}$ ) in the permeate was lower than the influent content ( $48 \text{ mgPO}_4\text{-P}\cdot\text{L}^{-1}$ ).  
6 These results indicated that uncontrolled P precipitation processes were taking place  
7 inside the reactor, which had already been observed by several authors (Barat et al., 2009;  
8 Doyle and Parsons, 2002; Martí et al., 2017), who reported precipitation problems in the  
9 digestion stage of a WWTP when treating sludges coming from biological removal  
10 processes. For that reason, a mass P balance was applied to the anaerobic digester,  
11 considering the average influent and effluent concentrations to estimate the potential P-  
12 recovery. P balances were based on the organic P ( $P_{\text{org}}$ ) content per gram of VSS  
13 ( $\text{gP}_{\text{org}}\cdot\text{gVSS}^{-1}$ ) in microalgae and primary sludge substrates. This  $P_{\text{org}}$  content was  
14 calculated as the difference between total P concentration and phosphate concentration in  
15 each substrate. The experimental values showed a content of  $0.010 \text{ gP}_{\text{org}}\cdot\text{gVSS}^{-1}$  for  
16 primary sludge and a content of  $0.013 \text{ gP}_{\text{org}}\cdot\text{gVSS}^{-1}$  for microalgae. These values were in  
17 agreement with those observed in the literature:  $0.013 \text{ gP}_{\text{org}}\cdot\text{gVSS}^{-1}$  in primary sludge  
18 were reported by Martí et al. (2008) and  $0.011 \text{ gP}_{\text{org}}\cdot\text{gVSS}^{-1}$  in microalgae were reported  
19 by González-Camejo et al. (2020). Two mass balances were carried out (Table 4)  
20 according to the value of biodegradability used for the calculations. The first balance  
21 considered the biodegradability percentage described in Section 3.1 (62.5%) obtained  
22 digesting microalgae and primary sludge substrates together (combined  
23 biodegradability). The second balance was calculated according to the biodegradability  
24 percentage obtained when digesting each substrate alone (separated biodegradability).  
25 This was 54% for microalgae digestion and 55% for primary sludge (data not shown).



1 Both mass balances showed similar results, revealing meaningful phosphate precipitation  
 2 (Table 4). Around  $35 \text{ mgP} \cdot \text{d}^{-1}$  were fixed in the reactor, representing 74% of the available  
 3 phosphorus. Only 26% of the phosphate was available for recovery in the effluent (Table  
 4 4). Influent and effluent calcium and magnesium concentrations (data not shown)  
 5 indicated a calcium and magnesium precipitation of around 11 and 7%, respectively. This  
 6 cation precipitation along with the high ammonium concentration suggests the formation  
 7 of struvite or other phosphate compounds inside the reactor. Uncontrolled P precipitation  
 8 hindered the recovery of phosphate through a struvite precipitation process after the AD  
 9 step, reducing potential P recovery in the treatment plant. Nevertheless, a significant  
 10 proportion ( $145 \text{ mg P} \cdot \text{L}^{-1}$ ) was recovered in the biosolids fraction.

11 **Table 4.** Mass phosphate balances carried out in the anaerobic digester using separated  
 12 and combined biodegradability. Average values and standard deviation are shown.

	Separated BD	Combined BD
$\mathbf{gP_{org} \cdot gVSS^{-1}_{influent}}$	$0.011 \pm 0.007$	$0.011 \pm 0.007$
$\mathbf{P_{loss} (gP \cdot kg \text{ sludge}^{-1})}$	$7.3 \pm 0.7$	$7.4 \pm 0.7$
$\mathbf{P_{available} (gP \cdot kg \text{ sludge}^{-1})}$	$2.7 \pm 0.6$	$2.5 \pm 0.6$
$\mathbf{Potential P_{recovery} (\%)}$	$27.4 \pm 6.5$	$25.5 \pm 6.1$

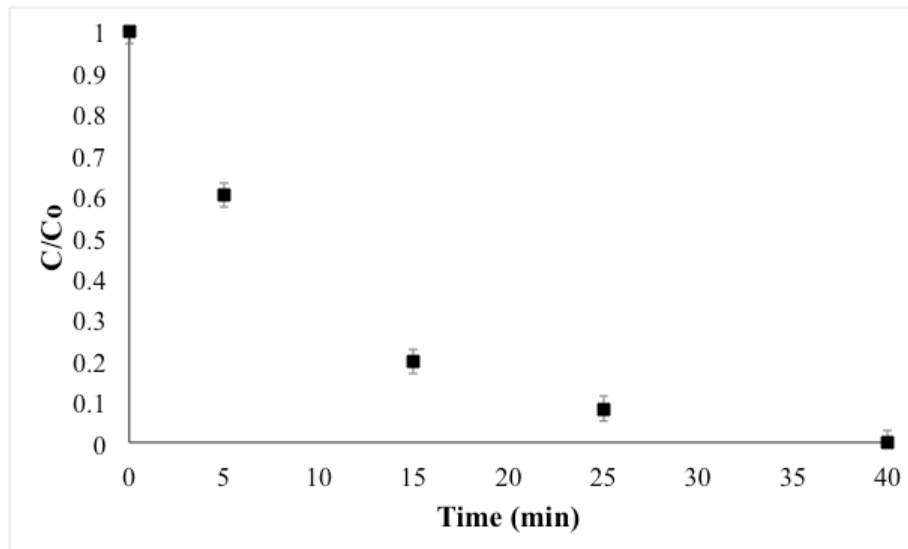
13 *BD: biodegradability* *P<sub>loss</sub>: phosphorus precipitated; P<sub>available</sub>: phosphorus available for recovery.*

14 Although there was not enough P in the ACoD plant permeate to apply struvite  
 15 precipitation, the TAN content could be recovered from the permeate. N recovery  
 16 processes are usually applied after P recovery, mainly struvite precipitation, in which pH  
 17 is raised and some cations and P precipitate. This precipitation process leads to non-  
 18 settleable solids formation (fine carbonate and phosphate precipitated particles), which  
 19 must be separated in order to avoid membrane clogging. In this case, as no struvite  
 20 precipitation step was performed, pre-treatment was needed at the beginning of the

1 process to raise the pH and later separate the solids formed. In these steps some N (around  
2 15% w/w) was lost by stripping, which is a similar value to that reported by Noriega-  
3 Hevia et al. (2020). The results obtained applying HFMC to the ACoD permeate showed  
4 a recovery efficiency of 99% in an operating time of approximately 40 min. Figure 2  
5 shows the TAN evolution during the experiment. TAN concentration first dropped by  
6 40% after 5 min, reaching the maximum recovery rate because of the high concentration  
7 of FAN in the ACoD permeate. As the flux is closely related to the FAN concentration,  
8 which decreased, the recovery rate was slowly reduced until complete TAN recovery.  
9 Due to the FAN passing through the membrane, the pH in the feed solution storage tank  
10 decreased, so that during the experiment the pH had to be maintained at a value of around  
11 10 by adding sodium hydroxide in order to maintain all TAN as FAN and consequently  
12 the driving force. The overall mass transfer coefficient calculated applying Eq. 4 was  
13  $1.4 \cdot 10^{-6} \text{ m} \cdot \text{s}^{-1}$ , which is similar to the values obtained by Noriega-Hevia et al., (2020) and  
14 Kartohardjono et al., (2015). The overall  $\text{NH}_4$ -flux obtained in the experiment was 0.4  
15  $\text{kgNH}_4\text{-N} \cdot \text{m}^{-2} \cdot \text{h}^{-1}$ , being  $1.43 \text{ kgNH}_4\text{-N} \cdot \text{m}^{-2} \cdot \text{h}^{-1}$  the highest flux obtained at the beginning  
16 of the experiment, when the FAN concentration was higher. The higher the N  
17 concentration in the feed solution, the lower the membrane required per kg of N  
18 recovered.

19 The product obtained at the end of the experiment was an ammonia sulphate solution with  
20 a maximum N richness of 4%, which is similar to that obtained by Richter et al., (2019)  
21 in a full scale plant. This ammonia sulphate solution is an inorganic salt that could be  
22 used as a substitute for the currently used chemical fertilisers. The market price of the  
23 sulphate solution obtained can vary,  $2.3 \text{ €} \cdot \text{kg N}^{-1}$  being the value proposed by Dube et al.,  
24 (2016).

25



**Figure 2.** Total ammonium nitrogen (TAN) evolution during hollow-fibre membrane contactor tests. C is the TAN concentration and C<sub>0</sub> is the initial TAN concentration

Although the N recovery results through HFMC are promising for future scaling up of the technology, a previous economic analysis would be required. Membrane costs are among the highest costs of the technology. For instance, the membrane cost in the present work is estimated at 0.5 € per cubic meter of N-rich stream treated. Since this stream (rejected water from sludge dewatering) usually represents less than 5% of the flow rate entering the WWTP, the membrane cost would be less than 0.025 €·m<sup>-3</sup> of influent wastewater. Nevertheless, a detailed economic study, including the market price of the sulphate solution and the main costs of the technology (membrane and chemical reagents) is required. An optimization study is also needed to obtain the operating conditions to minimize operating costs prior to full-scale implementation of this technology.

### 3.3 Composting performance

The composting process of ACoD sludge and conventional sludge was evaluated. The effect of applying different aeration modes, adding inoculum to the mixtures and mixing sludge with BA in different proportions was assessed. To determine whether the

1 composting process and stabilisation were achieved, mixtures from each reactor were  
2 characterised at the beginning (Initial characterisation) and end of the process (Final  
3 characterisation) (Table 5).

4 The initial characterisation showed that C/N ratio was in general lower in mixtures  
5 generated with volumetric proportions since there was a higher content of sludge in those  
6 mixtures. TN content was higher in reactors containing ACoD sludge ( $R_{A1}$ ,  $R_{A2}$  and  $R_{A3}$ )  
7 than in their replicates containing conventional sludge ( $R_{C1}$ ,  $R_{C2}$  and  $R_{C3}$ ) due to the  
8 presence of microalgae biomass in the ACoD process, which has a high nitrogen content  
9 (Ullah et al., 2015). Initial moisture content was around 60% in all the reactors, which is  
10 an optimum value to start the composting process (Bueno et al., 2008; Diaz and Savage.,  
11 2007). Reactors containing ACoD sludge had higher moisture content associated with the  
12 dewatering method used for each sludge. It is also remarkable that in the mixtures  
13 containing volumetric proportions, moisture content was higher than in the ones  
14 generated by theoretical calculations due to their higher proportion of sludge (Table 5),  
15 except in reactor  $R_{A1}$ . Initial porosity was in general higher in mixtures with theoretical  
16 proportions of sludge and BA. Initial pH and electrical conductivity of both types of  
17 mixture had typical initial values according to their ACoD and conventional sludge  
18 composition. The initial characterisation of ACoD sludge (not mixed with BA or  
19 inoculum) showed no presence of *E. coli* neither *Salmonella* spp., microorganisms  
20 commonly used as pathogen indicators. Conventional sludge (not mixed with BA or  
21 inoculum) contained no *Salmonella* spp. but was positive for *E. coli*. The inoculum added  
22 to some reactors (Table 2) also showed the presence of *E. coli*.

1 **Table 5.** Initial and final characterisation of the composting samples for the seven reactors evaluated.

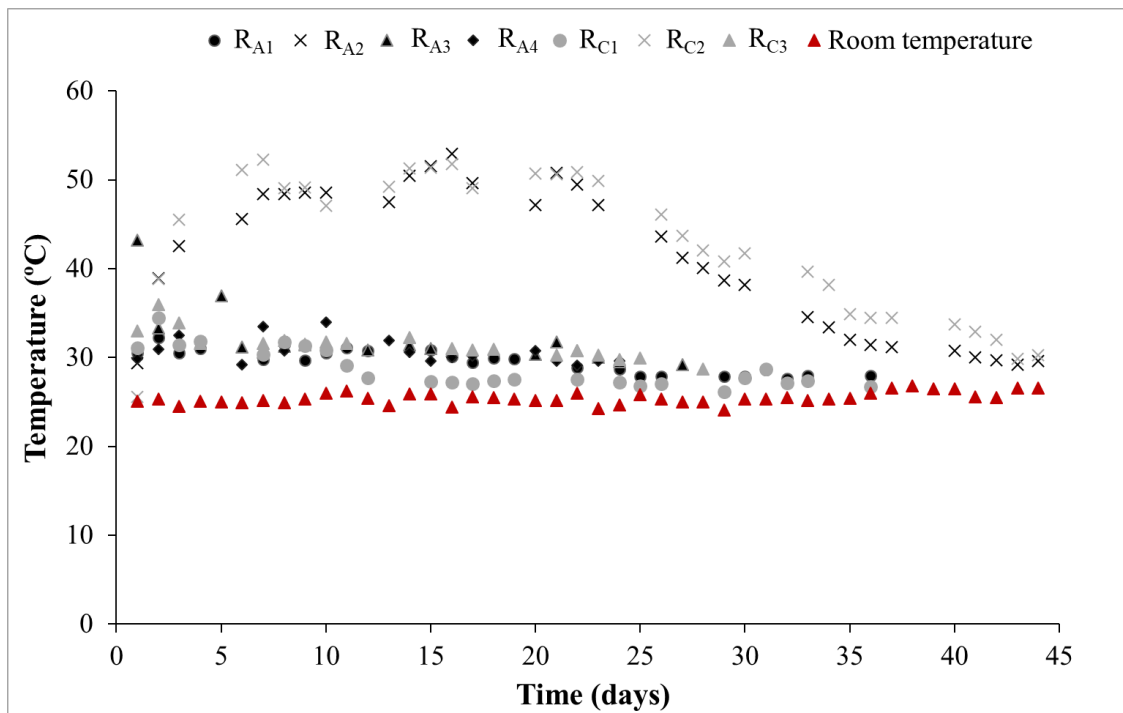
	<i>ACoD sludge</i>				<i>Conventional sludge</i>		
	<b>R<sub>A1</sub></b>	<b>R<sub>A2</sub></b>	<b>R<sub>A3</sub></b>	<b>R<sub>A4</sub></b>	<b>R<sub>C1</sub></b>	<b>R<sub>C2</sub></b>	<b>R<sub>C3</sub></b>
Mixtures	Theoretical	v/v	Theoretical	v/v	Theoretical	v/v	Theoretical
Aeration / Inoculum	Forced / Yes	Turned / Yes	Turned / Yes	Turned / No	Forced / Yes	Turned / Yes	Turned / Yes
<b>Initial characterisation</b>							
C/N ratio	24.0 ± 0.7	14.5 ± 0.1	15.8 ± 0.5	16.4 ± 0.1	24.6 ± 0.1	12.5 ± 0.1	19.1 ± 0.1
Moisture content (%)	68.1 ± 0.8	68.7 ± 0.4	59.5 ± 0.9	62.9 ± 0.4	58.6 ± 0.5	64.3 ± 0.7	50.4 ± 0.5
O.M (%)	87.2 ± 0.6	84.1 ± 0.5	75.8 ± 0.9	87.0 ± 0.6	80.1 ± 0.8	86.9 ± 0.5	75.9 ± 0.5
Porosity (%)	36.5 ± 0.9	13.4 ± 0.9	38.4 ± 0.6	21.6 ± 1.5	33.4 ± 2.0	24.0 ± 1.4	40 ± 1.0
TN (gN·kg <sub>d.m.</sub> <sup>-1</sup> )	15.0 ± 1.2	24.6 ± 1.1	15.1 ± 1.3	9.6 ± 0.7	11.6 ± 0.9	11.5 ± 0.8	9.1 ± 0.6
pH	7.8 ± 0.1	7.4 ± 0.1	8.1 ± 0.6	6.3 ± 0.1	8.5 ± 0.0	8.5 ± 0.1	8.5 ± 0.1
Conductivity (μS·cm <sup>-1</sup> )	1104 ± 158	1418 ± 272	1775 ± 144	2620 ± 190	1905 ± 247	1490 ± 258	1992 ± 249
<i>E. coli</i> *	A	A	A	A	P	P	P
<i>Salmonella</i> spp.*	A	A	A	A	A	A	A
<b>Final characterisation</b>							
C/N ratio	14.9 ± 0.0	13.0 ± 0.2	14.9 ± 0.0	13.2 ± 0.3	13.7 ± 0.2	11.4 ± 0.1	17.6 ± 0.2
C/N ratio removal (%)	37.9	10.3	5.4	19.5	44.3	8.8	7.8
Moisture content (%)	68.3 ± 0.3	70.0 ± 1.0	61.6 ± 0.5	67.96 ± 0.6	47.8 ± 0.2	67.8 ± 0.5	66.5 ± 0.4
O.M (%)	76.6 ± 0.2	75.8 ± 0.7	74.8 ± 0.3	82.51 ± 0.5	74.8 ± 0.5	66.8 ± 0.9	72.5 ± 0.3
O.M. removal (%)	12.1	9.8	1.3	5.2	6.5	23.1	4.6
pH	8.3 ± 0.1	8.7 ± 0.1	8.3 ± 0.1	7.8 ± 0.2	8.0 ± 0.1	8.2 ± 0.2	8.11 ± 0.05
Conductivity (μS·cm <sup>-1</sup> )	1908 ± 196	2300 ± 253	1990 ± 181	3010 ± 173	2170 ± 260	3530 ± 224	2800 ± 250
<i>E. coli</i>	P	A	P	P	P	A	P
<i>Salmonella</i> spp.	A	A	A	A	A	A	A

2 *O.M.*: Organic matter; *TN*: Total nitrogen; *d.m.*: dry matter; *v/v*: volumetric proportions; *A*: absence of microorganisms in sludge; *P*: presence of microorganisms in sludge;  
 3 \*' pathogens in initial samples were only measured in the sludge, not in the mixtures of sludge and bulking agent.

1 Final characterisation of the samples (Table 5) showed that moisture content in general  
2 did not significantly change from the initial conditions. However, reactor R<sub>C1</sub> lost a  
3 significant amount of moisture, from 59% to 48%, associated with an excess of forced  
4 aeration that contributed to the dryness of the material. On the other hand, reactor R<sub>C3</sub> had  
5 increased moisture content, from 50% to 66%. This could be explained by inadequate  
6 drainage of the excess moisture. pH values were around 8, which are in agreement with  
7 typical values of mature compost (8.0 – 8.5 (Diaz and Savage, 2007)). Conductivity  
8 values showed an increase in comparison to initial values in all reactors (Table 5).  
9 According to Diaz and Savage (2007) this behaviour is typical in a composting process  
10 due to the mineralisation of the organic matter. All the samples from the reactors (except  
11 for R<sub>A2</sub> and R<sub>C2</sub>) contained *E. coli* after composting. Since the initial samples from the  
12 ACoD reactors were *E. coli* free (Table 5) and the initial characterisation of the inoculum  
13 showed *E. coli* to be present, it is possible that they were contaminated by the inoculum.  
14 Only reactors R<sub>A2</sub> and R<sub>C2</sub> achieved complete sanitation after the composting process,  
15 showing a final material without *E. coli* and *Salmonella* spp.

16 With regard to temperature evolution, reactors R<sub>A2</sub> and R<sub>C2</sub>, with volumetric mixtures,  
17 showed temperatures between 50 °C and 53 °C achieving the thermophilic temperature  
18 necessary for the sanitation of the composted material (Insam and De Bertoldi, 2007). In  
19 the case of reactor R<sub>A2</sub>, the thermophilic phase was reached during a period of 20 days  
20 and in reactor R<sub>C2</sub> during a period of 24 days (Figure 3). Reactor R<sub>A2</sub>, which contained  
21 ACoD sludge, had a larger acclimatisation period since it followed the same evolution as  
22 reactor R<sub>C2</sub> (with conventional sludge) but with a delay of several days (Figure 3). This  
23 could be associated with a lower content of easily biodegradable organic matter due to  
24 the high SRT (70 d) of the previous ACoD process. In both reactors upper organisms such  
25 as mites were found as bioindicators of the correct progress of the composting process

1 (Soliva, 2001). Indeed, these two reactors also showed no presence of *E. coli* and  
 2 *Salmonella* spp., as mentioned in the paragraph above. The remaining reactors did not  
 3 achieve thermophilic temperatures probably due to: the excess of aeration in reactors with  
 4 forced aeration ( $R_{A1}$ ,  $R_{C1}$ ) (Bueno et al., 2008 and Negro et al., 2000 reported that aeration  
 5 should not be excessive to avoid inhibiting microbial activity), coupled with the small  
 6 volumes of the reactors, which led to higher heat losses; the absence of inoculum in  
 7 reactor  $R_{A4}$  (Manu et al., 2017 observed how the composting process could be improved  
 8 by adding a microbial inoculum); and/or the lower sludge content in the reactors prepared  
 9 with theoretical proportions ( $R_{A1}$ ,  $R_{C1}$ ,  $R_{A3}$ ,  $R_{C3}$ ).



10

11 **Figure 3.** Evolution of mixture temperature and room temperature during composting  
 12 period.

13 Regarding the effect of applying different aeration modes, the results indicated that forced  
 14 aeration offers better composting conditions, since a higher organic matter and C/N ratio

1 removal was observed in reactors R<sub>A1</sub> and R<sub>C1</sub>, compared with their replicates R<sub>A3</sub> and  
2 R<sub>C3</sub>, which were aerated by hand (Table 5).

3 The results regarding the effect of adding an inoculum source to the sludge-BA mixtures  
4 indicated that inoculum has an important effect on composting since thermophilic  
5 temperature and sanitation were achieved in reactor R<sub>A2</sub> but not in reactor R<sub>A4</sub>. Higher  
6 organic matter removal (9.8%) was also observed in reactor R<sub>A2</sub> than in R<sub>A4</sub> (5.2%).

7 Finally, regarding the effect of mixing sludge and BA in different proportions the results  
8 indicated that reactors R<sub>A2</sub> and R<sub>C2</sub>, which were prepared with volumetric proportions of  
9 BA and ACoD and conventional sludge, respectively, achieved thermophilic temperature  
10 and showed complete sanitation of the compost, suggesting that this mixture method  
11 should be used for composting. Both these reactors were aerated by hand and achieved  
12 percentages of organic matter removal and C/N ratio removal of 9.8 and 10.3 for reactor  
13 R<sub>A2</sub> and 23.1 and 8.8 for R<sub>C2</sub>, respectively. In contrast, reactors R<sub>A3</sub> and R<sub>C3</sub>, run under  
14 the same operating conditions but mixed with theoretical proportions did not achieve  
15 thermophilic temperature and showed percentages of organic matter removal and C/N  
16 ratio removal of 1.3 and 5.4 for reactor R<sub>A3</sub> and 4.6 and 7.8 for R<sub>C3</sub>, respectively, being  
17 these percentages lower than the ones observed for the pair of reactors R<sub>A2</sub> and R<sub>C2</sub> (Table  
18 5).

19 When comparing all the reactors, of those fed with conventional sludge, R<sub>C2</sub> showed the  
20 best performance in terms of compost sanitation, temperature achieved and organic matter  
21 removal efficiency, but not in terms of C/N ratio removal (Table 5). Reactor R<sub>C1</sub> showed  
22 the best C/N ratio removal (44%) but did not achieve thermophilic temperature or  
23 compost sanitation. Of the reactors fed with ACoD sludge, even R<sub>A2</sub> showed the best  
24 performance in terms of compost sanitation and temperature achieved, organic matter and  
25 C/N ratio removal were not among the highest values obtained from all the reactors. In



1 fact,  $R_{A1}$  showed an organic matter and C/N ratio removal percentage of 12.1 and 37.9,  
2 respectively, this being the highest value for these two parameters in those reactors fed  
3 with ACoD sludge (Table 5). The two reactors mentioned ( $R_{A1}$  and  $R_{C1}$ ) were prepared  
4 with mixtures calculated by theoretical proportions with forced aeration. The previous  
5 results suggest that, although reactors  $R_{A2}$  and  $R_{C2}$  were the only ones that achieved  
6 compost sanitation, and the volumetric proportions seemed to be more suitable, a higher  
7 organic matter and C/N ratio removal could be achieved by forced aeration in ACoD  
8 sludge reactors at the correct airflow to achieve the best composting conditions.

9 In general, it can be observed that the higher the initial C/N ratio, the greater the  
10 elimination of this parameter, which is the case of mixtures generated with theoretical  
11 proportions. However, in mixtures generated with volumetric proportions, the amount of  
12 sludge added to the mixtures is 3.2 times higher than in the theoretical ones. Therefore, it  
13 actually contains more biodegradable organic matter but it also has a much higher N  
14 content, which makes the initial C/N ratio lower. In addition, ammonia may have been  
15 lost due to the combination of ammonification of the organic nitrogen and the basic pH  
16 values reached during the composting process. The latter may result in a not significant  
17 change of the initial values of C/N ratio for mixtures generated with volumetric  
18 proportions and thus in lower C/N ratio removal. Therefore, since C/N ratio is a chemical  
19 composting parameter and not a biochemical one, it could be assumed that this alone is  
20 not a suitable indicator of the process evolution and that all the parameters (aeration,  
21 temperature, moisture content, presence of inoculum, proportions of sludge and BA)  
22 should be controlled. New BA and sludge ratios should thus be applied in future  
23 experiments to achieve correct C/N ratio removal and material sanitation. The optimum  
24 aeration rate flow should also be studied to obtain the optimum composting results.

25

1 3.3.1. Respirometry: monitoring the biological activity

2 Biological activity was only measured in the reactors that reached thermophilic  
3 temperatures, since this is an indicator of correct process evolution and higher biological  
4 activity in the mixture. Respirometric tests were therefore carried out in reactors R<sub>C2</sub>  
5 (conventional sludge) and R<sub>A2</sub> (ACoD sludge). The RIs at process temperature and 37 °C  
6 are shown in Table 6 for each respirometric test performed.

7 **Table 6.** Respirometric index (RI) obtained in respirometric tests.

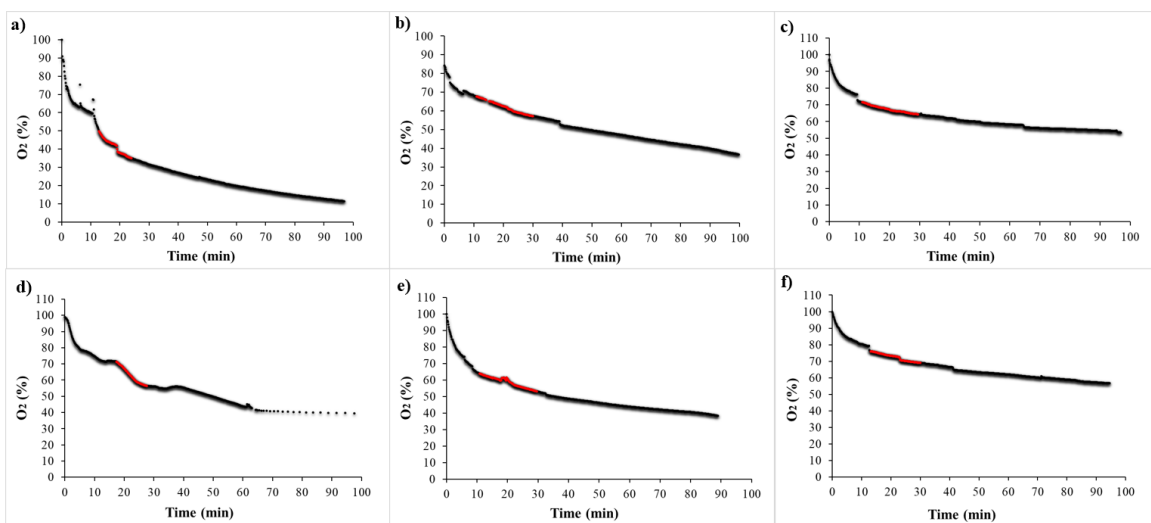
Respirometric test	Day of composting	RI (mgO <sub>2</sub> ·gO.M. <sup>-1</sup> ·h <sup>-1</sup> )
RI <sub>A2</sub> (48°C)	7	3.28
RI <sub>A2</sub> (52°C)	15	1.81
RI <sub>A2</sub> (44°C)	26	1.51
RI <sub>C2</sub> (51°C)	6	4.46
RI <sub>C2</sub> (52°C)	16	2.04
RI <sub>C2</sub> (44°C)	27	1.72
RI <sub>C2</sub> (37°C)	44	0.242
RI <sub>A2</sub> (37°C)	44	0.257

8 *O.M.: organic matter*

9 Figures 4a, 4b and 4c show the biological activity measurement for reactor R<sub>A2</sub> on Days  
10 7, 15 and 26 of operations, which were Days 2, 10 and 21 at the beginning of the  
11 thermophilic phase, respectively. Figures 4d, 4e and 4f show the measurement of the  
12 biological activity for reactor R<sub>C2</sub> on Days 6, 16 and 27 of operation, corresponding to  
13 Days 4, 14 and 25 at the beginning of the thermophilic phase, respectively.

14 The slope of the curves marked in red colour in Figure 4 represents the oxygen uptake  
15 rate. The higher the composting time, the lower the slope of the curve. RI thus decreased  
16 as composting time increased, so that biological activity also decreased, the first few days  
17 of the thermophilic phase being the period with the highest biological activity. R<sub>C2</sub>

1 showed the same behaviour as  $R_{A2}$ . The reactor containing ACoD sludge ( $R_{A2}$ ) presented  
 2 lower RI than  $R_{C2}$  at the same temperature (Table 6). This could be explained by the  
 3 higher SRT (70 d) in the previous ACoD process than in the conventional AD (SRT of  
 4 20 d), which led to a lower biodegradable substrate concentration available for  
 5 microorganisms, therefore lower biological activity. The thermophilic phase was also  
 6 longer in  $R_{C2}$  than in  $R_{A2}$  (Figure 3), which also indicates higher biological activity in the  
 7 mixture.



8  
 9 **Figure 4.** Oxygen percentage evolution over time at process temperature in reactors  $R_{A2}$   
 10 (a, b, c) and  $R_{C2}$  (d, e, f).

11 The respirometric tests at 37 °C showed that the lower the RI, the more stable the mixture.  
 12 At the end of the composting process, the biological activity decreased significantly in  
 13 both reactors, showing similar RI values (Table 6). As established by the TMECC (US  
 14 Department of Agriculture and Council, 2001), a composted material becomes stabilised  
 15 when RI at 37 °C is between 0.5-1.5  $\text{mgO}_2 \cdot \text{g organic matter}^{-1} \cdot \text{h}^{-1}$ , and is very stable when  
 16 RI is below 0.5  $\text{mgO}_2 \cdot \text{g organic matter}^{-1} \cdot \text{h}^{-1}$ , is in this case. In terms of stability, the  
 17 mixtures in reactors  $R_{C2}$  and  $R_{A2}$  were therefore stabilised (Table 6).

1 Composting after a previous AD step was thus achieved from both ACoD and  
2 conventional sludge. Reactors  $R_{A2}$  and  $R_{C2}$ , prepared with volumetric proportions,  
3 achieved thermophilic temperatures and complete compost sanitation. Reactor  $R_{C2}$  had  
4 higher organic matter removal (2.4-fold higher) but lower C/N ratio removal (1.1-fold  
5 lower) than  $R_{A2}$ . Both reactors showed RI values associated with a stabilised composted  
6 material and the respirometric tests indicated that the process temperature RI fell as  
7 composting time increased.

#### 8 **4 CONCLUSIONS**

9 Three potentially useful by-products were generated through microalgae and primary  
10 sludge co-digestion in an AnMBR: methane-rich biogas, nitrogen-rich permeate and  
11 nutrient-rich digestate. Nitrogen was recovered from the permeate at 99% efficiency and  
12 an ammonia sulphate solution, which could be used as a commercial fertiliser, was  
13 obtained. For the first time, composting process applied to a digestate coming from a  
14 microalgae co-digestion plant was evaluated in the present work at laboratory scale.  
15 ACoD digestate composting was compared to a conventional AD digestate composting,  
16 and similar values were obtained for both. The best composting performance in terms of  
17 sanitation of the composted material and removal of organic matter and C/N ratio was  
18 obtained when mixing sludge with BA in volumetric proportions (2.5 volume of BA per  
19 1 volume of sludge), applying forced aeration and adding an inoculum from an industrial  
20 compost plant to accelerate the biological process. Respirometric tests indicated a highly  
21 stable final compost. The combination of microalgae co-digestion with subsequent  
22 composting offers complete resource recovery (energy, nutrients and water) from sewage  
23 in a circular economy-based scenario for future WRRF implementation.

24

## 1 **CRedit authorship contribution statement**

2 Rebecca Serna-García: Conceptualization, Methodology, Investigation, Formal analysis,  
3 Writing - Original draft. Patricia Ruiz-Barriga: Methodology, Writing – Original draft.  
4 Guillermo Noriega-Hevia: Formal analysis, Writing – Original draft. Joaquín Serralta:  
5 Writing- Review & Editing. María Pachés: Supervision; Writing – Review & Editing.  
6 Alberto Bouzas: Supervision, Writing – Review & Editing, Funding acquisition.

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