An analytical model for Tensairity girders

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Abstract

An analytical model for the deflection of a symmetric, spindle shaped Tensairity girder under homogenous load is proposed which can be solved analytically. The results are compared to FEM predictions for a specific Tensairity girder. Further simplifications of the analytical solution lead to a simple relation for the deflection of the Tensairity girder, which reveals the importance of the elasticity of the chords relative to the air pressure in the inflated hull. Such simple models are crucial to understand the basic principles of Tensairity and provide the engineer with easy rules to estimate the load-deflection behaviour of this new light weight structure.

Keywords: Tensairity, inflatable structures, bending, analytical models

1. Introduction

Tensairity is a new structural concept where an inflated hull is combined with compression and tension members in a synergetic way (Luchsinger *et al.* [1]). First applications in civil engineering such as roof and bridge structures have been successfully realised with Tensairity. Recently, intensive research was conducted with Tensairity structures. Luchsinger and Crettol [2] have subjected spindle shaped Tensairity girders to local bending loads and the deformation behaviour was compared to FEM predictions. Plagianakos *et al.* [3] have investigated spindle shaped Tensairity columns subjected to axial compressive loads. In both studies, measurements were made at different air pressure levels as a major focus of these investigations was the influence of the air pressure on the structural behaviour of Tensairity. An increase in stiffness with increasing pressure was observed with a trend to saturation for higher pressure values. Recently, the role of fabric webs inside Tensairity columns was investigated by Wever *et al.* [4].

In order to understand the structural behaviour of Tensairity, simple analytical models are of great importance. Such a model was proposed by Wever *et al.* [4] for the stiffness of a Tensairity column under axial compression. Based on the solution of a 4'th order ordinary differential equation, a simple relation for the axial displacement as a function of the slenderness of the spindle, the span, the chord stiffness and the air pressure was deduced. Initial work for bending of Tensairity girders was conducted by Huguenot [5]. Here we propose an analytical model for the behaviour of symmetric spindle shaped Tensairity girders subjected to homogeneous bending load. The topic is elaborated in more detail in an ongoing PhD thesis by Teutsch [6].



Figure 1: Basic set up of the spindle shaped Tensairity girder.

2. Analytical model for symmetric spindle shaped Tensairity girders

The construction of the spindle shaped Tensairity girder is shown in Figure 1. The girder consists of an upper and a lower parabolic chord, which are separated by a spindle shaped inflated hull. The chords are assumed to be connected to the hull. The central idea is to model the inflated hull of the Tensairity structure as an elastic foundation for the chords with the modulus of the foundation being a function of the air pressure (Plagianakos *et al.* [3]). In this model, we assume the bending stiffness of the chords to be very small so that it can be neglected. The homogeneous load acts on the upper chord leading to a deformation of the chord and the inflated hull and thus to a load transfer to the lower chord. Thus, the girder is described by two coupled equations for the deflections of the upper and lower chords w_1 and w_2 respectively.

$$H \cdot \frac{d^2}{dx^2} (z_1 + w_1) - G \cdot \frac{d^2}{dx^2} w_1 + k \cdot (w_1 - w_2) = q$$
(1a)

$$-H \cdot \frac{d^2}{dx^2} (z_2 + w_2) - G \cdot \frac{d^2}{dx^2} w_2 - k \cdot (w_1 - w_2) = 0$$
(1b)

where *H* is the horizontal force component, *G* the shear stiffness of the elastic foundation, *k* the modulus of the elastic foundation and *q* the homogenously distributed load. The shear stiffness *G* is assumed to be constant. The modulus of the foundation is proportional to the air pressure *p* and given for the problem at hand by (Luchsinger *et al.* [1])

$$k = \frac{\pi \cdot p}{2} \tag{2}$$

The two chords have a parabolic shape

$$z_1 = -f \cdot \left(1 - \left(\frac{x}{l}\right)^2\right) \quad , \quad z_2 = f \cdot \left(1 - \left(\frac{x}{l}\right)^2\right) \tag{3}$$

Solving Eq. (1a) for w_2 and by insertion in Eq. (1b) one obtains the 4th order differential equation

$$\frac{d^4 w_1}{dx^4} - \frac{2 \cdot G \cdot k}{(G^2 - H^2)} \cdot \frac{d^2 w_1}{dx^2} = \frac{4 \cdot k \cdot \Delta H \cdot f}{(G^2 - H^2) \cdot l^2}$$
(4)

with

$$\Delta H = H - H_0 \tag{5}$$

$$H_0 = \frac{q \cdot l^2}{4 \cdot f} \tag{6}$$

Enforcing symmetry (w(-x)=w(x)) and the boundary conditions $w_1(l) = w_2(l) = 0$, one finds

$$w_{1} = \frac{C_{0} \cdot \cosh(\lambda \cdot x)}{\lambda^{2}} + \frac{C_{1} \cdot x^{2}}{2} + C_{3}$$
(7)

$$w_2 = C_0 \cdot \left(\frac{H-G}{k} + \frac{1}{\lambda^2}\right) \cdot \cosh(\lambda \cdot x) + C_1 \cdot \left(\frac{H-G}{k} + \frac{x^2}{2}\right) + C_3 - \frac{q}{k} + \frac{H \cdot 2 \cdot f}{k \cdot l^2}$$
(8)

$$C_{0} = -\frac{C_{1}}{\cosh(\lambda \cdot l)} + \frac{q \cdot l^{2} - 2 \cdot H \cdot f}{(H - G) \cdot l^{2} \cdot \cosh(\lambda \cdot l)} , \qquad C_{1} = \frac{\Delta H \cdot 2 \cdot f}{G \cdot l^{2}}$$
(9)

$$C_3 = C_1 \cdot \left(\frac{1}{\lambda^2} - \frac{l^2}{2}\right) - \frac{q \cdot l^2 - 2 \cdot H \cdot f}{(H - G) \cdot l^2 \cdot \lambda^2} \qquad , \qquad \lambda = \sqrt{\frac{2 \cdot G \cdot k}{G^2 - H^2}}$$

The horizontal force H is determined by the constraint that the horizontal displacement of the ends of the two chords has to be identical

$$\frac{H \cdot l}{E \cdot A} - \frac{2 \cdot f}{l^2} \cdot \int_0^l w_1 \cdot dx = \frac{2 \cdot f}{l^2} \cdot \int_0^l w_2 \cdot dx - \frac{H \cdot l}{E \cdot A}$$
(10)

with E the Young's modulus and A the cross sectional area of the two identical chords. The integrals are evaluated to

$$\int_{0}^{l} w_{1} \cdot dx = \frac{C_{0} \cdot \sinh(\lambda \cdot l)}{\lambda^{3}} + \frac{C_{1} \cdot l^{3}}{6} + C_{3} \cdot l$$
(11)

$$\int_{0}^{l} w_2 \cdot dx = \frac{C_0 \cdot \sinh(\lambda \cdot l)}{\lambda} \cdot \left(\frac{H - G}{k} + \frac{1}{\lambda^2}\right) + C_1 \cdot l \cdot \left(\frac{H - G}{k} + \frac{l^2}{6}\right) + C_3 \cdot l - \frac{q \cdot l}{k} + \frac{2 \cdot H \cdot f}{k \cdot l} \quad (12)$$

Eq. (10) is solved numerically in iterative steps.

3. Example: Deformation of a Tensairity spindle with 5m span

As an example we consider a symmetric spindle shaped Tensairity column with 5 m span (l = 2.5m) and a slenderness 10 corresponding to f = 0.25m. The rectangular cross section of the two identical aluminum chords is 3cm x 1cm (EA = 20.7 MN). Such a girder was

experimentally and numerically studied under point load by Luchsinger and Crettol [2]. Loads up to 500 N/m and air pressure values up to 450 mbar are considered.

In order to calculate w_1 and w_2 , the shear stiffness of the elastic foundation needs to be specified. In lack of a model we assume that it is proportional to the pressure. The pressure dependent part of the shear stiffness of an airbeam is $G = p \cdot S$ with p the air pressure and S the cross sectional area of the beam (Topping [7]). Adopting this model, we set $G = p \cdot \pi \cdot f^2$ neglecting the variation of the cross sectional area along the length of the spindle.

In Figure 2, w_1 and w_2 are shown for p = 150 mbar and q = 200 N/m. For comparison, predictions of a detailed FEM model of the spindle including material properties of the hull as well as the bending stiffness of the chords are shown. In view of the approximations of the analytical model, a remarkable correspondence between the analytical results and the FEM results is found both for w_1 and w_2 .



Figure 2: Analytical deflection of the chords compared to FEM results.

The influence of G on the deformation has been investigated by a parameter study. As it turns out both w_1 and w_2 do depend only very weakly on G provided that H < G. For H = G the parameter λ (Eq. (9)) has a singularity and for H > G a complex number is found for λ leading to undulating deflections patterns for w_1 which are not observed in experiments.

Thus, one might state that the proposed model is only valid for $H \le G$ while the exact value for G is not critical. Nevertheless, the true nature of G in Tensairity needs to be investigated in more detail especially in combination with the bending stiffness of the chords. This is subject of further studies.

4. Simplified analytical model

For the Tensairity girder of the example above, further simplifications can be made by a detailed study of the various terms of Eqs. (7) and (8). One finds that the deformation at mid span can be approximated as

$$w_1(0) \approx C_3 \tag{13}$$

$$w_2(0) \approx C_3 - \frac{q}{k} + \frac{H_0 \cdot 2 \cdot f}{k \cdot l^2} \approx C_3 - \frac{q}{2 \cdot k}$$
 (14)

with $H \approx H_0$. As can be seen from Figure 2, w_1 and w_2 are fairly constant along the length of the spindle. Thus, the integrals can be approximated as

$$\int_{0}^{l} w_{1} \cdot dx \approx C_{3} \cdot l \tag{15}$$

$$\int_{0}^{l} w_2 \cdot dx \approx C_3 \cdot l - \frac{q \cdot l}{2 \cdot k}$$
(16)

and Eq. (10) can be solved for C_3 . As a final result, one finds

$$w_1(0) = \frac{1}{4} \cdot \varepsilon \cdot \gamma \cdot L + \frac{1}{4} \cdot \frac{q}{k}$$
(17)

$$w_2(0) = \frac{1}{4} \cdot \varepsilon \cdot \gamma \cdot L - \frac{1}{4} \cdot \frac{q}{k}$$
(18)

where the span L=2l, the strain ε and the slenderness γ have been introduced

$$\varepsilon = \frac{H_0}{E \cdot A}, \qquad \gamma = \frac{L}{2 \cdot f} \tag{19}$$

For the Tensairity girder of the example one obtains $w_1(0) = 2.9$ mm and $w_2(0) = -1.4$ mm from Eqs. (17) and (18), which is very close to the result of the full analytical model as shown in Fig. 2. In the simple model the deflections are independent of G which confirms that G has not a strong impact on w_1 and w_2 as long as it is larger than H.

Defining the stiffness of the compression chord as

$$m_1 = \frac{q \cdot L}{w_1(0)} \tag{20}$$

one obtains with Eq. (17)

$$m_1 = \frac{4}{L \cdot \left(\frac{\gamma^2}{8 \cdot E \cdot A} + \frac{1}{k \cdot L^2}\right)}$$
(21)

The stiffness of the Tensairity spindle for the simple analytical model (Eq. (21)) and the FEM results are shown in Figure 3 as a function of the air pressure. The simple model slightly underestimates the results of the FEM calculation, however, the correspondence is remarkable in view of all the approximations and simplifications in the simple model.



Figure 3: Stiffness of the 5m Tensairity spindle under bending load as a function of air pressure.

The two summands in the denominator of m_1 are equal for

$$p_b = \frac{16 \cdot E \cdot A}{\pi \cdot \gamma^2 \cdot L^2} \tag{22}$$

corresponding to a pressure of 430 mbar for the girder at hand. Thus, for air pressure values smaller than p_b , the deformation of the upper chord is dominated by the deformation of the elastic foundation while for higher pressure values the elasticity of the chords is dominant.

4. Conclusions

An analytical model is proposed for symmetric spindle shaped Tensairity girders subjected to homogenous bending load. The inflated body of the Tensairity structure is modeled as an elastic foundation with shear stiffness. The bending stiffness of the chords is neglected. The two coupled ordinary differential equations can be solved analytically. It is found that the shear stiffness of the elastic foundation is very important to stabilize the compression chord, however, its exact value is not very important as long as it is larger than the horizontal force. A comparison of the analytical deflections with FEM results shows a good agreement for a specific Tensairity spindle. The analytical model is further simplified by neglecting minor terms. The deflection at mid span is found to be the sum of two terms, one due to the elasticity of the chords and one due to the deformation of the inflated body which depends on the air pressure. The stiffness given by the simple model is found to be in good agreement with FEM prediction for various air pressure values. Further studies will include the role of the bending stiffness of the chords, the exact nature of the shear stiffness of the elastic foundation and detailed comparisons with experimental data.

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