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Additional Information

1 **Management of table olive processing wastewater by an osmotic membrane**  
2 **bioreactor process**

3

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6

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11

12 **Keywords:** Forward osmosis, Osmotic membrane bioreactor, Wastewater treatment.

13

14

15 **Abstract**

16 The management of fermentation brines from the table olive processing is very complex  
17 due to its characteristics: high salinity and high organic matter concentration including  
18 phenolic compounds, which behave as slow degradable compounds when a biological  
19 process is performed. In this work, the management of these effluents by an osmotic  
20 membrane bioreactor has been assessed. This technique combines a biological reactor  
21 with forward osmosis membranes. For the study, a laboratory plant consisting of 1 L  
22 reactor and a forward osmosis module equipped with a membrane of 42 cm<sup>2</sup> of active  
23 surface has been used. Fermentation brine from table olive processing was fed to the  
24 system both as draw solution to set out the driving force for the membrane process and  
25 as a part of the feed to the reactor, mixing it with municipal wastewater. The

26 experiments were carried out at a constant feed to microorganism ratio of 0.4 g COD·g  
27 SS<sup>-1</sup>·d<sup>-1</sup>. Results indicated that the hypersaline effluent was able to produce the needed  
28 driving force by the process. Permeate fluxes ranged between 1 and 1.5 L·m<sup>-2</sup>·h<sup>-1</sup> after  
29 the flux decay of the first operation days. Concerning the biological reaction, it has to  
30 be highlighted that phenols were eliminated after 24 days. Until that day, the  
31 biological process was jeopardized due to the quick increase of the conductivity in the  
32 reactor (ranging between 30 and 35 mS·cm<sup>-1</sup>), which was caused not only by the  
33 salinity of the influent but also by the reverse salt flux phenomenon. Soluble microbial  
34 products and extracted extracellular polymeric substances also increased in the reactor  
35 during the start-up.

36

37

## 38 **1. Introduction**

39

40 The treatment of hypersaline wastewaters containing organic matter (such as those from  
41 food-processing, leather or petroleum industries) [1] is very complex, requiring further  
42 researches and new economic and environmental-friendly solutions. One of these  
43 hypersaline effluents is produced in the table olive processing, which is one of the major  
44 industrial activities in Mediterranean countries such as Spain, Italy or Greece [1].  
45 According to the data published [2], the table olive production worldwide during the  
46 2017/2018 season was more than 2900 tonnes. Table olive processing includes several  
47 steps: rinsing with fresh water, debittering in an alkaline solution, a second rinsing with  
48 water and a final fermentation in a NaCl brine [3]. Around 6 L·kg<sup>-1</sup> of table olive  
49 processing wastewaters are generated in the overall process [4]. Although wastewater  
50 generated in the fermentation step only involves the 20% of the total volume generated,

51 the wastewater from fermentation of table olive processing (FTOP) produces the 85%  
52 of the total wastewater pollution [5]. The main characteristics of the FTOP are high  
53 salinity (conductivity around  $80 \text{ mS}\cdot\text{cm}^{-1}$ ) and high organic matter concentration due to  
54 oil and greases, phenolic compounds and volatile organic acids [6]. However, these  
55 characteristics clearly depend on the olive maturity, cultivar type, season and  
56 fermentation time [7,8].

57 Although it could be thought that these wastewaters can be treated effectively by means  
58 of several techniques such as evaporation or reverse osmosis, the high content in  
59 organic matter make the use of these technologies especially complicated. In this  
60 context, the possibility of offering environmental-friendly solutions for this wastewater  
61 acquires relevant importance.

62

63 In the recent years, the appearance of novel forward osmosis (FO) membranes has  
64 increased the potential applications of this technique. Specifically, osmotic membrane  
65 bioreactors (OMBR) offer the possibility of a simultaneous degradation of the organic  
66 matter in the biological reactor and a dilution of the hypersaline wastewater as a  
67 consequence of its use as draw solution. An OMBR is an emerging technology  
68 combining FO membranes with a biological reactor [9]. A typical OMBR consists of a  
69 FO membrane module, a biological reactor and a draw solution (DS) reservoir. The DS  
70 creates a high osmotic pressure difference, which acts as a driving force, between the  
71 both membrane sides. As a consequence, the treated water is transported from the  
72 biological reactor through the FO membrane to the DS (what implies the dilution of the  
73 DS) [10,11]. Compared to conventional membrane bioreactors (MBR), OMBR presents  
74 lower membrane fouling degree, higher fouling reversibility and significant lower  
75 energy consumption since is not hydraulically driven membrane technology [12]. In

76 addition, OMBRs offer higher water quality due to the high rejection capacity of FO  
77 membrane to pathogens [13], ions [14], and organic compounds [15]. One potential  
78 application of OMBRs is for eliminating slowly degradable compounds from industrial  
79 wastewaters. In this way, Praveen et al., [16] studied the biodegradation of phenol  
80 compounds from saline wastewater using an OMBR. The results indicated that the  
81 treatment was effective and the membrane fouling was reversible.

82

83 However, the main shortcoming in the OMBR operation is the salinity build-up in the  
84 biological reactor, which is associated with the reverse salt flux and with the high  
85 rejection capacity of the FO membranes [17,18]. Salt accumulation in the bioreactor has  
86 negative effects on the overall process efficiency since the osmotic driving force  
87 decreases and salinity build-up can also affect the physical and biochemical  
88 characteristics of the microorganisms [19]. As a consequence, the membrane water flux  
89 is reduced and the activated sludge could increase the production of soluble microbial  
90 products and extracellular polymeric substances (EPS), leading to their concentration in  
91 the bioreactor, which implies higher membrane fouling [19].

92

93 In this research, a laboratory OMBR was operated to study the suitability of the process  
94 both for the treatment of FTOP as a part of the bioreactor feed and for its dilution as a  
95 DS for the FO process. The process was studied in terms of treated water quality  
96 (paying special attention to elimination of phenols), membrane water flux, salinity  
97 accumulation in the reactor, mixed liquor characteristics and membrane fouling. Until  
98 now, this alternative for the FTOP treatment has not been reported in the bibliography.

99

100

101 **2. Materials and methods**

102

103 2.1. Experimental system

104 The process flow diagram of the OMBR used for the experiments is shown in Fig. 1.

105 The FO flat sheet membrane module CF042-FO, which was supplied by Sterlitech

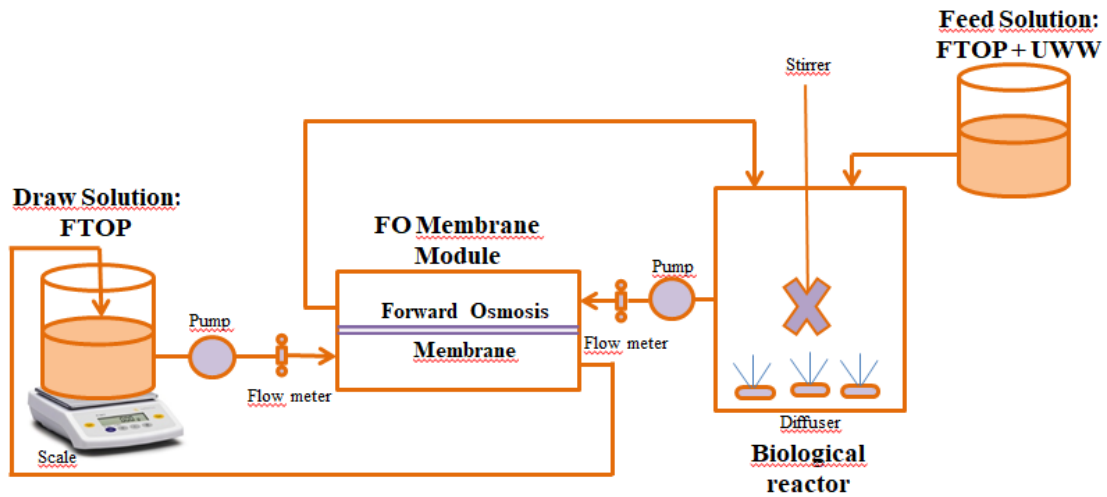
106 (USA), was coupled with a laboratory reactor. This module has the capacity for testing

107 one membrane with an effective area of 42 cm<sup>2</sup>. The FO commercial membrane

108 employed was CTA-NW membrane from Hydration Technology Innovations (HTI,

109 USA). Further details of the laboratory OMBR can be found in earlier works[20].

110



111

112 **Figure 1: Process flow diagram of the experimental pilot plant.**

113

114 2.2. Wastewaters characterization

115 The influent to the OMBR, consisted of FTOP and urban wastewater mixed in different

116 proportions according to the strategy detailed in Section 2.3. FTOP was provided by a

117 company located in Valencia (Spain). Each sample of 5-10 liters was filtered in a 60 μm

118 sieve in order to reduce the suspended solids concentration and was stored at 4°C before

119 its use. FTOP was characterized in terms of pH, conductivity, chloride (Cl<sup>-</sup>), sodium

120 (Na<sup>+</sup>), soluble total nitrogen (TN), soluble total phosphorous (TP), soluble chemical  
121 oxygen demand (COD) and total phenolic compounds. pH and conductivity were  
122 measured using a pH-Meter GLP 21<sup>+</sup> and EC-Meter GLP 31<sup>+</sup> (Crison, Spain),  
123 respectively. Na<sup>+</sup> was analyzed by means of ion selective digital probe ISENa 381  
124 (Hach, USA). Cl<sup>-</sup>, TN, TP and COD were measured using kits from Merck. Finally, the  
125 total phenolic compounds concentration was measured following the Folin-Ciocalteu  
126 method [21]. In this way, the total phenolic content (expressed as tyrosol concentration,  
127 mg TY·L<sup>-1</sup>) was measured spectrophotometrically. Sodium carbonate (20% w/v) from  
128 Panreac, Folin&Ciocalteu's reagent and Tyrosol analytical standards, from Sigma  
129 Aldrich, were used in the analytical procedure.

130

131 The urban wastewater was characterized measuring soluble COD, TN and TP using kits  
132 also from Panreac (Spain). Table 1 and 2 show the characteristics of each wastewater  
133 used. As it can be observed in Table 1, FTOP has high conductivity values due to the  
134 NaCl used in the fermentation brine process [22]. However, due to its wide variability,  
135 sodium chloride addition was required (until reaching a conductivity value of 80  
136 mS·cm<sup>-1</sup>) to the first and third FTOP samples in order not to reduce the osmotic  
137 pressure difference in the process.

138

139

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**Table 1: Samples of FTOP used for the experiments.**

	<b>Experiment day</b>		
	0-9	10-48	49-62
<b>Conductivity</b> (mS·cm <sup>-1</sup> )	34.4	82.1	16.46
<b>pH</b>	5.52	3.74	4.82
<b>Cl (mg·L<sup>-1</sup>)</b>	13,200	44,700	5,700
<b>COD (mg·L<sup>-1</sup>)</b>	8,050	25,850	4,500
<b>TP (mg·L<sup>-1</sup>)</b>	22.8	70	24.5
<b>TN (mg·L<sup>-1</sup>)</b>	138	220	120
<b>Na (mg·L<sup>-1</sup>)</b>	11,000	35,000	3,975

145

146

147

**Table 2: Samples of urban wastewater used for the experiments.**

	<b>Experiment day</b>			
	0-6	7-27	28-44	45-62
<b>COD (mg·L<sup>-1</sup>)</b>	167	113	147	184
<b>TP (mg·L<sup>-1</sup>)</b>	2.1	1.8	4	7.1
<b>TN (mg·L<sup>-1</sup>)</b>	26	7	48	41
<b>pH</b>	7.12	6.85	6.70	7.05

148

149

150 On the other hand, the low COD values of the urban wastewater samples were typical  
 151 for the wastewater treatment plant where the samples were taken. It has also to be  
 152 commented that the ratio C/N/P was checked in the mixed wastewater feeding the  
 153 OMBR to avoid a lack of nutrients in the reactor. Thus, dipotassium phosphate (from  
 154 Panreac, Spain) and urea (from Panreac, Spain) were employed to adjust the  
 155 phosphorous and nitrogen concentrations to ensure a COD/N/P relationship of 100/5/1  
 156 mg·L<sup>-1</sup>, since the FTOP had low phosphorous and nitrogen concentrations (as it is  
 157 shown in Table 1).

158



159 2.3. Experimental protocol

160 The OMBR was inoculated with activated sludge from a municipal wastewater treatment  
161 plant (located in Valencia, Spain). The food to microorganisms ratio (F/M) was fixed to  
162  $0.4 \text{ g COD} \cdot \text{g SS}^{-1} \cdot \text{d}^{-1}$ . To this purpose, FTOP and urban wastewater were mixed in the  
163 appropriate proportions to achieve a COD around  $2,600 \text{ mg} \cdot \text{L}^{-1}$  since the flow treated  
164 per day was fixed by the membrane water flux at  $0.3 \text{ L} \cdot \text{d}^{-1}$ . The total duration of the  
165 OMBR operation was 60 days. The initial pH and conductivity values were 7.13 and  
166  $1.65 \text{ mS} \cdot \text{cm}^{-1}$ , respectively.

167

168 The reactor was fed 8 times per day, by adding the same wastewater volume as the  
169 permeated water volume through the FO membrane.

170

171 The osmotic backwashing was carried out twice per week to clean the FO membrane.  
172 In this process, deionized water was used as DS and a NaCl solution with a  
173 concentration of  $70 \text{ g} \cdot \text{L}^{-1}$  as FS. Both solutions were recirculated through the system  
174 during 1.5 hours. In addition, every 20 days, the membrane backwashing was  
175 followed by a chemical cleaning step. This step was carried out by using a solution of  
176 1% w/w of Alconox (from Alconox, United States) and 0.8% w/w of EDTA (from Alfa  
177 Aeser, United States) recirculated during 2 hours. It is important to note that after each  
178 membrane cleaning step, a fresh DS was introduced for the OMBR operation.

179

180 2.4. Analytical methods

181

182 2.4.1. Membrane characterization

183 On one hand, the pristine FO membrane was characterized measuring the membrane  
184 water flux and RSF following the methodology described in a previous study [23].

185

186

187

188

#### 189 2.4.2. Mixed liquor characterization

190 To study the concentration of the biomass, MLSS and mixed liquor volatile suspended  
191 solids (MLVSS) concentrations in the biological reactor were measured twice a week  
192 following the procedure described in [24]. In addition, pH was measured also twice a  
193 week.

194

195 To analyze the filterability of sludge, capillary suction time (CST) was measured once a  
196 week using the equipment 304M from Triton Electronics Ltd (United Kingdom). The  
197 results were normalized against MLSS concentrations and the result was expressed in  
198 units of  $s \cdot L \cdot gMLSS^{-1}$ .

199

200 Finally, chemical characteristics of the sludge in terms of Extracellular Polymeric  
201 Substances (EPS) and Soluble Microbial Products (SMP) were studied. The extracted  
202 EPS were obtained by means of a cation exchanger resin (Dowex Marathon C, Sigma  
203 Aldrich, Spain) following the procedure described in [25]. eEPS and SMP in the  
204 biological reactor were studied in terms of proteins and carbohydrates. Proteins  
205 concentrations were analyzed using Bicinchonnic acid (BCA) assay test from Novagen.  
206 Carbohydrates were analyzed following the Antrone method [26]. Furthermore, DNA  
207 was quantified using Quant-it<sup>TM</sup> dsDNA HS (0.2-100 ng) kit from Invitrogen (Spain).

208

#### 209 2.4.3. Reactor performance

210 In order to study the OMBR performance in terms of organic matter, phenolic  
211 compounds and nutrients removal, the soluble fraction of the biological reactor was  
212 analyzed to check the quality of the treated water.

213

### 214 **3. Results**

215

#### 216 3.1. Properties of the CTA NW FO membrane

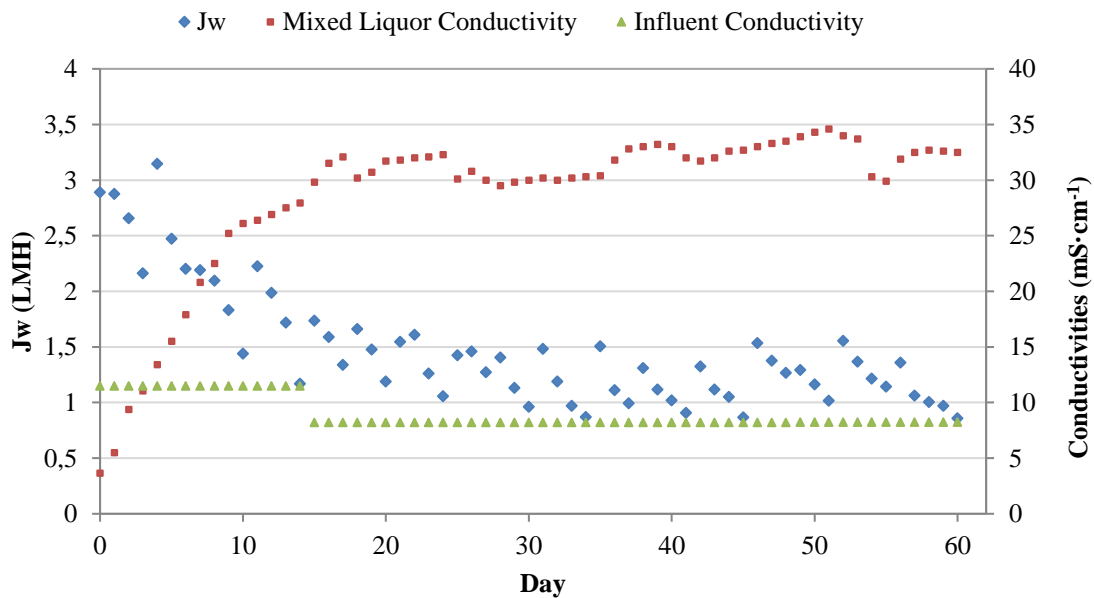
217 Membrane water fluxes ( $J_w$ ) and reverse salt flux ( $J_s$ ) were measured for different  
218 concentrations of NaCl. Thus, the normalized salt reverse flux ( $J_s/J_w, g \cdot L^{-1}$ ) was of 0.58  
219  $g \cdot L^{-1}$  as mean value of the data obtained for NaCl concentrations of 25, 100 and 200  
220  $g \cdot L^{-1}$ . These results are in agreement with previous findings [27,28]. For instance,  
221 Siddique et al.[29] published a normalized salt reverse flux value of 0.75 using NaCl  
222 with a concentration of 14  $g \cdot L^{-1}$  as DS. Moreover, according to Luo et al. [30] who also  
223 tested this FO membrane, the contact angle and zeta potential of this membrane were  
224 60.4° and -4.5 mV, respectively.

225

#### 226 3.2. Water flux and salinity build-up in the bioreactor

227 Salinity build-up in the bioreactor is a critical issue during the OMBR operation mainly  
228 due to the high salt rejection of the FO membrane and the reverse salt flux from the DS  
229 [31]. As it can be observed in Fig. 2, salinity increased dramatically in the bioreactor  
230 from 3.66 to 29.8  $mS \cdot cm^{-1}$  within the first 15 days of OMBR operation. The severe  
231 increase can be attributed mostly to the high salinity of the influent in this period  
232 (reactor feed was the mixture of FTOP and urban wastewater, as describe in materials

233 and methods section). In addition, the higher membrane water flux in that period  
 234 indicated the lower membrane fouling degree, what implied higher reverse solute flux.  
 235 By contrast, from the 15<sup>th</sup> day of operation, the conductivity of the influent was lower  
 236 than the reactor conductivity and the membrane water flux was also lower due to the  
 237 reduction of the osmotic pressure difference. Furthermore, the sludge withdrawals to  
 238 maintain the MLSS concentration also contributed to hinder the end of the salinity  
 239 increase in the reactor. This confirms that in this type of processes a maximum salinity  
 240 value in the reactor is being established. For instance, Raghavan et al. [32] treated  
 241 synthetic wastewater with an OMBR and observed a final value of the biological reactor  
 242 conductivity of 27.9 mS·cm<sup>-1</sup>. Nevertheless, [33] Luo et al. also studied the treatment of  
 243 synthetic wastewater with an OMBR and reached a final mixed liquor conductivity  
 244 value of 11.5 mS·cm<sup>-1</sup>.  
 245



246  
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 250

Figure 2: Water flux and mixed liquor conductivity evolution.

251 FO membrane water flux decreased continuously during the OMBR operation. This  
252 trend is due to membrane fouling and salinity build-up in the bioreactor [34,35] and it is  
253 in agreement with the results reported by Luo et al. [33]. Membrane water flux  
254 evolution can be divided into two stages: the first one is until the 10<sup>th</sup> day of OMBR  
255 operation and the second one related to the last part of the experiment. In the first stage,  
256 the membrane water flux significantly decreased from 3.1 LMH to 1.4 LMH. This fact  
257 could be attributed to the fast decrease in the osmotic pressure difference between both  
258 sides of the membrane and, for a less extent, to the deposition of foulants on the  
259 membrane surface [36]. In the second stage (from the day 10<sup>th</sup> of operation time), the  
260 fouling layer was already formed and a low membrane water flux, being almost  
261 constant, was observed. These results are in concordance with previous studies  
262 performed with laboratory OMBRs [23,32,37].

263

264

### 265 3.3. Pollutants removal performance

266

#### 267 3.3.1. Organic matter removal

268 The high COD value of the FTOP made difficult to check that no organic matter had  
269 permeated through the FO membrane from the FS to the DS. Anyway, it was calculated  
270 the theoretical COD of the diluted DS considering the volume increment in the DS  
271 reservoir; this result was then compared with the analysed COD. Results indicated that  
272 COD removal efficiencies in the OMBR were around 100%. Other authors like  
273 Raghavan et al.[32] and Morrow et al.[38] also published high total organic carbon  
274 removal efficiencies of 98% and 98.4%, respectively, operating an OMBR.

275 Concerning the soluble COD in the reactor, it was increasing until it reached an  
276 approximately constant value of  $900 \text{ mg}\cdot\text{L}^{-1}$  at 35<sup>th</sup> day. This means that after an  
277 adaption period, no accumulation of COD occurred in the reactor. In other words, the  
278 increase of the organic matter biodegradation counteracted the concentration effect of  
279 the FO membrane.

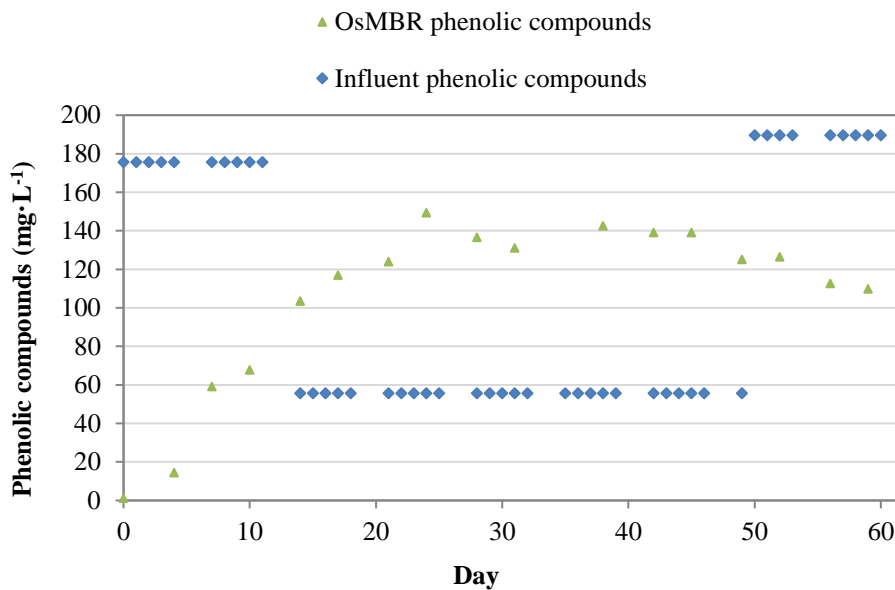
280

### 281 3.3.2. Removal of phenolic compounds

282 Fig. 3 illustrates the OMBR and influent concentrations of total phenolic compounds  
283 during the total duration of the experiment. Phenolic compounds increased rapidly  
284 during the first 20 days of operation independently of the influent concentration, being  
285 the maximum value detected  $149.41 \text{ mg}\cdot\text{L}^{-1}$  at the day 24<sup>th</sup> of operation.

286 The variation of the total phenols concentration with time can be divided into 4 periods.  
287 The first period coincides with the feeding of the first sample to the reactor. Due to its  
288 high total phenols concentration the accumulation of these compounds in the reactor  
289 was very quick. The second period starts with the feeding of the second sample, whose  
290 concentration of total phenols was considerably lower than the first sample, up to the  
291 24<sup>th</sup> day. Until that day, the total phenols concentration in the reactor increased  
292 gradually at a rate lower than in the first period until reaching the concentration of  
293  $149.41 \text{ mg}\cdot\text{L}^{-1}$ . From 24<sup>th</sup> to 45<sup>th</sup> day (third period), total phenolic compounds  
294 concentration was approximately constant (around  $140 \text{ mg/L}$ ), which means that  
295 biomass has been adapted to the salinity and the high phenols concentration and the  
296 biodegradation of these compounds began to occur. Finally, in the fourth period (last 10  
297 days of operation) the total phenols degradation rate increased in the reactor, and the  
298 concentration decreased until  $109.8 \text{ mg}\cdot\text{L}^{-1}$ .

299



300  
301 **Figure 3: OMBR and influent phenolic compounds evolution.**  
302

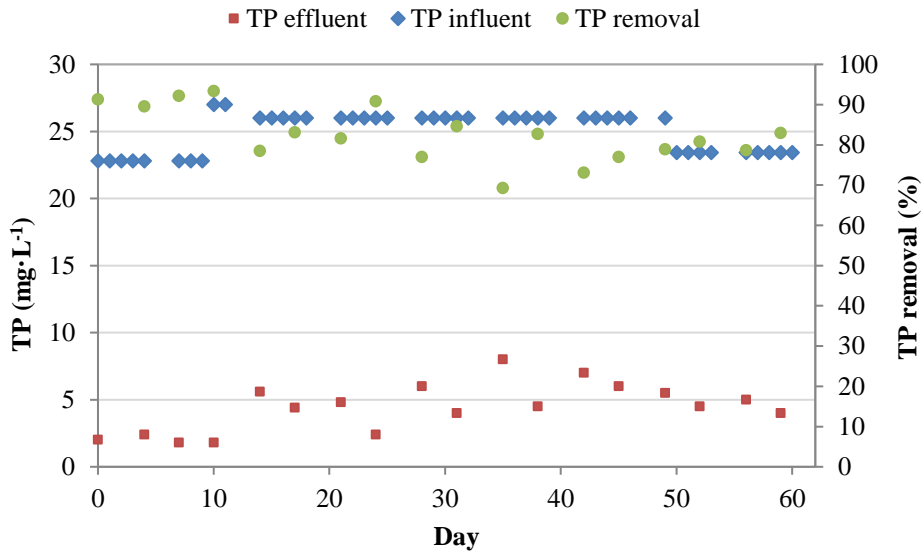
303 These results suggest that phenolic removal was performed through biological  
304 degradation [39]. Ferrer-Polonio et al., [40] published a total phenolic compounds  
305 biodegradation in a sequencing batch reactor (SBR) of around 76%, what implies that  
306 phenolic compounds biodegradation is really viable. However, these authors had to  
307 operate the reactor at very high hydraulic retention time (40 days).

308  
309 **3.4. Nutrients concentration in the reactor**

310 Total phosphorous was monitored by means of measuring the TP content in the  
311 supernatant and the TP concentration in the influent. As it can be observed in Fig. 4, the  
312 TP removal percentage varied between 69.23% and 93.33%, reaching a final value of  
313 82.92%. TP removal percentage slightly decreased until the day 35th of operation. Since  
314 the TP removal was due to assimilation by microorganisms for growth purposes, this  
315 trend was probably due to a mild deterioration of biomass.

316 TN concentrations were clearly affected by the nitrogen concentration in the wastewater  
317 used as FS. The evolution of the different nitrogen forms during the OMBR operation

318 can be observed in Fig. 5. The TN content in the supernatant was maintained at around  
 319  $40 \text{ mg}\cdot\text{L}^{-1}$  most of the operating days, excepting from day 28<sup>th</sup> until 35<sup>th</sup> during which  
 320 an increase in the  $\text{NH}_4\text{-N}$  concentration was observed. After that period, the  $\text{NH}_4\text{-N}$   
 321 consumption raised again, which corresponded to the increase of the COD biological  
 322 degradation as reported in Section 3.3.1. The average removal percentage of total  
 323 nitrogen was around 70% during the whole duration of the OMBR experiment.  
 324 However, this nitrogen elimination was only due to biomass assimilation since the low  
 325 TN concentrations in the influent required the addition of an external N source.  
 326



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Figure 4: Total phosphorous removal.



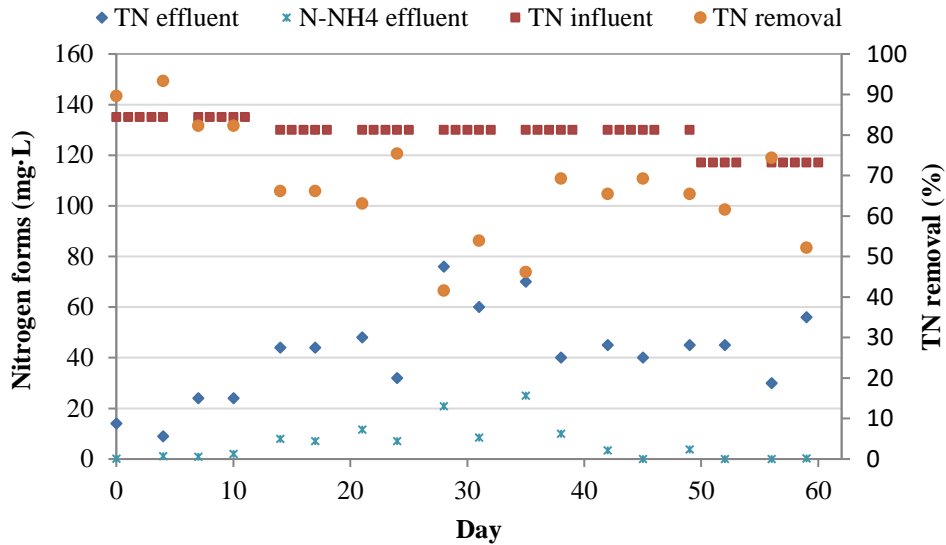


Figure 5: Nitrogen forms in the OMBR and TN removal.

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### 334 3.5. Mixed liquor characteristics

335

#### 336 3.5.1. Mixed liquor pH, MLVSS and normalized CST

337 Fig. 6 illustrated the mixed liquor pH, the percentage of MLVSS and the CST in the

338 mixed liquor during the OMBR experiment. Reverse draw solute flux can alter the

339 mixed liquor pH during the OMBR operation. Although both DS and FS used had a pH

340 lower than 7 (Tables 1 and 2), the pH slightly increased with the operating time of the

341 experiment reaching a final value of 8.8 (Fig. 6a).The reason of this pH increase can be

342 ascribed to the volatile acids (mainly lactic, malic, acetic and formic) contained in the

343 FTOP wastewater [41], which were oxidized to CO<sub>2</sub> and H<sub>2</sub>O by the biomass, thus

344 producing the pH increase [5].

345

346 Microorganisms growth can be altered by the salinity increase in the biological reactor

347 thus affecting the MLSS and MLVSS concentration [29]. As it can be observed in Fig.

348 6.a, the percentage of MLVSS decreased with time due to the increased osmotic stress  
349 on the feed side.

350

351 Sludge filterability in terms of CST was measured during the OMBR experiment (Fig.  
352 6.b). As capillary suction time increases, the filterability of the sludge decreases [29].

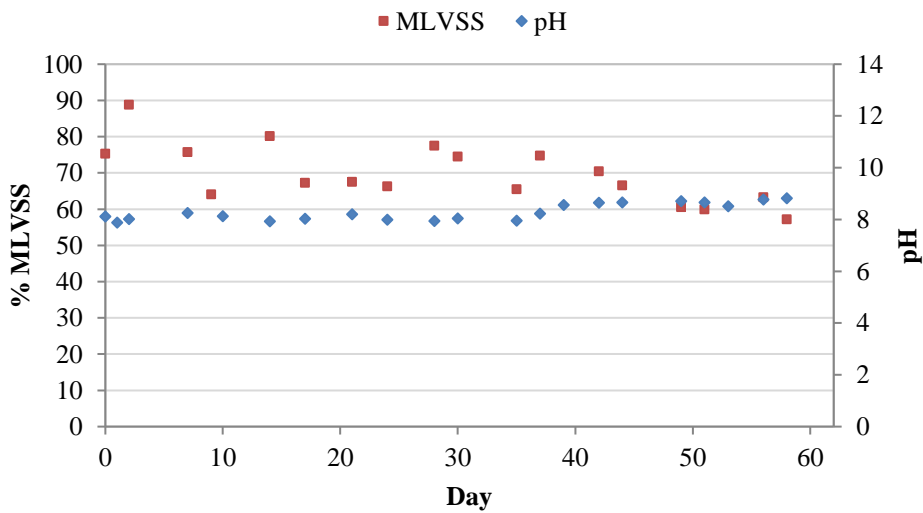
353 Deteriorated sludge filterability can be observed around the day 46<sup>th</sup> of operation, what  
354 coincided with the maximum mixed liquor conductivity. At the end of the experiment,

355 CST values reached lower values probably due to the diminution of the volatile solids  
356 percentage as explained above. These results are in concordance with previous studies

357 [42], which refer normalized CST values between 20.5 and 39.1 s·L·gTSS<sup>-1</sup> in the  
358 sludge of an OMBR for heavy metals removal[42].

359

360 a)



361

362 b)

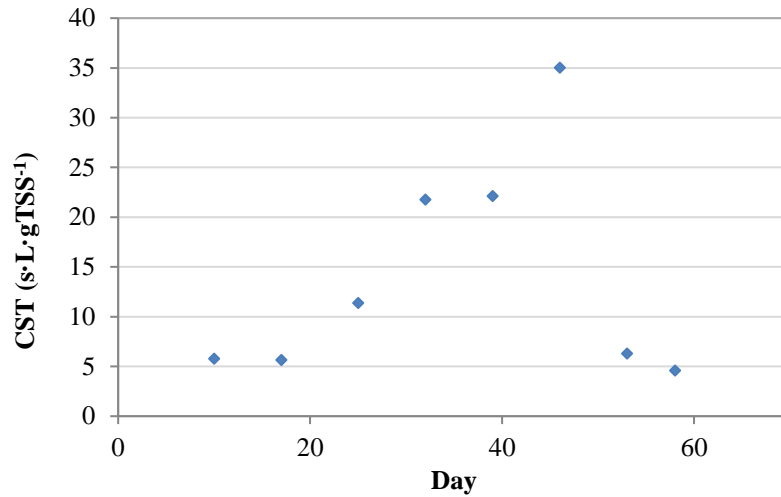


Figure 6: Mixed liquor characteristics a) **MLVSS** and b) CST.

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### 369 3.5.2. SMP and eEPS analysis

370 SMP and eEPS content in an OMBR operation has a paramount importance. Salinity

371 build-up in the biological reactor can increase the SMP and eEPS in the mixed liquor

372 mainly due to cell lysis and secretion of organic cellular substances[43]. Proteins and

373 carbohydrates are the main components found inherently in SMP and EPS [29]. SMP

374 and eEPS content play also an important role in the membrane fouling [44]. Fig. 7

375 shows SMP and EPS concentrations in terms of proteins (Figs. 7.a), carbohydrates (Fig.

376 7.b) and DNA (Fig. 7.c) during the experimental period. In Fig. 7.a and 7.b, it can be

377 observed that the SMP and eEPS concentrations increased as salt concentration

378 increased in the biological reactor. However, from the day 38<sup>th</sup> of operation on, SMP

379 and EPS concentrations decreased with the operation time. This denotes that biomass

380 adapted to salinity environment observed above. These results are consistent with

381 previously reported results, where salt accumulation in the mixed liquor resulted in an  
382 increased SMP and EPS content in an OMBR due to salinity accumulation [23,29,44].

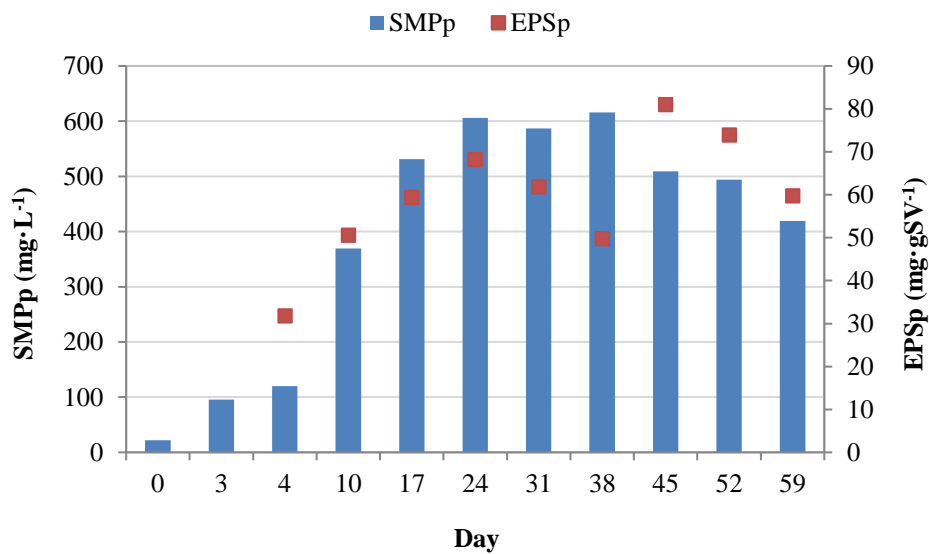
383

384 DNA has also been measured in the SMP in order to study the eventual cell lysis. As it  
385 can be observed in Fig. 7.c, DNA concentrations increased dramatically until 17<sup>th</sup> day  
386 due to cellular lysis caused by the increase of the salinity in the reactor. From that day  
387 on, the increase was very slight. That means that cellular lysis diminished and the  
388 sludge withdrawals could stop the DNA increase in the reactor.

389

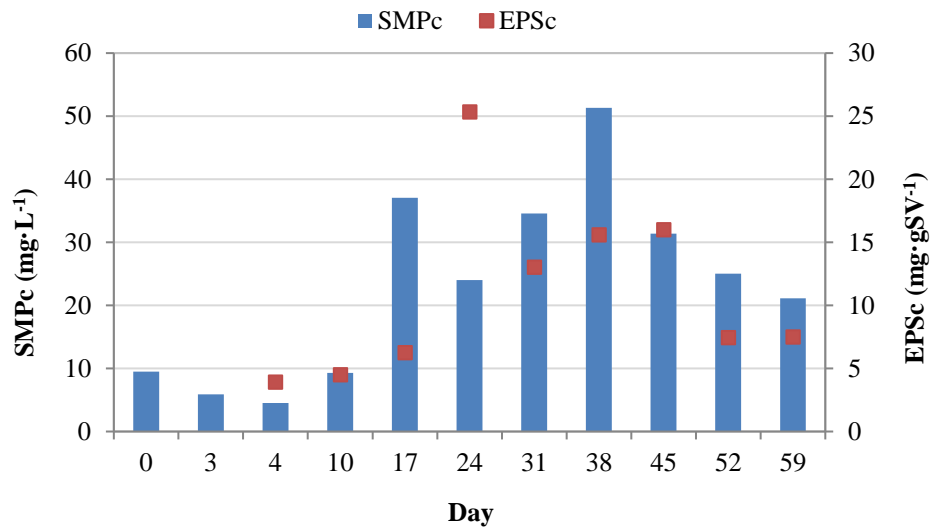
390

391 a)



392

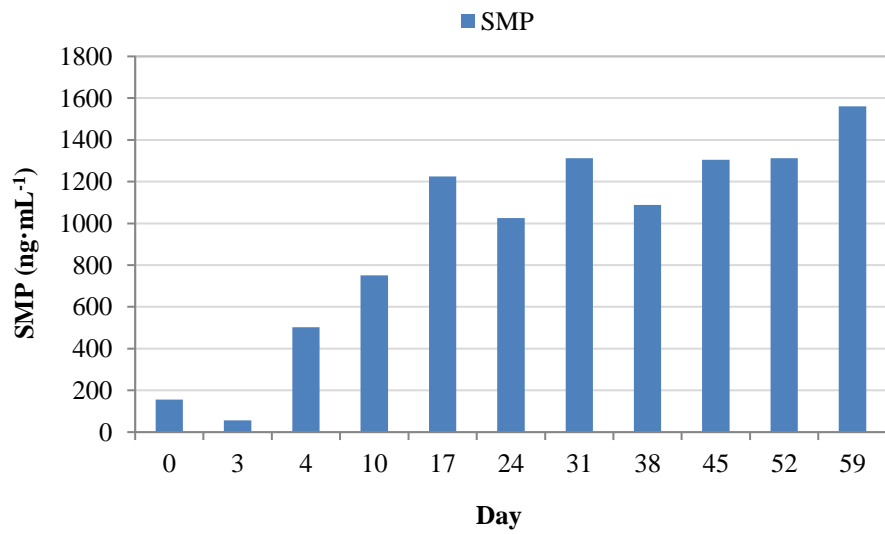
393 b)



394

395

396 c)



397

398 **Figure 7: SMP and EPS concentrations in terms of a) proteins, b) carbohydrates and c) DNA.**

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#### 405 **4. Conclusions**

406

407 The treatment of mixed municipal and industrial wastewaters has been successfully  
408 carried out with a laboratory OMBR. The residual fermentation brine from the table  
409 olive processing has produced the needed osmotic pressure difference between both  
410 sides of the membrane for the treated water permeation to the DS side. In this way, no  
411 costs for the regeneration of the DS have to be considered since the FTOP has been also  
412 used for this purpose. In addition, its feeding to the reactor mixed with municipal  
413 wastewater treatment has entailed the degradation of the phenols contained in the FTOP  
414 after an adaptation period.

415 On the other side, the salinity-build up, the generated SMP and the non-biodegradable  
416 organic matter accumulation have to be controlled in the process in order not to affect  
417 the biomass and consequently the organic matter removal efficiency.

418 The large-scale application of the OMBR to industrial wastewaters with slowly  
419 degradable organic matter will depend on the implementation of techniques for  
420 controlling the accumulation of the above mentioned substances.

421

422

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426

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