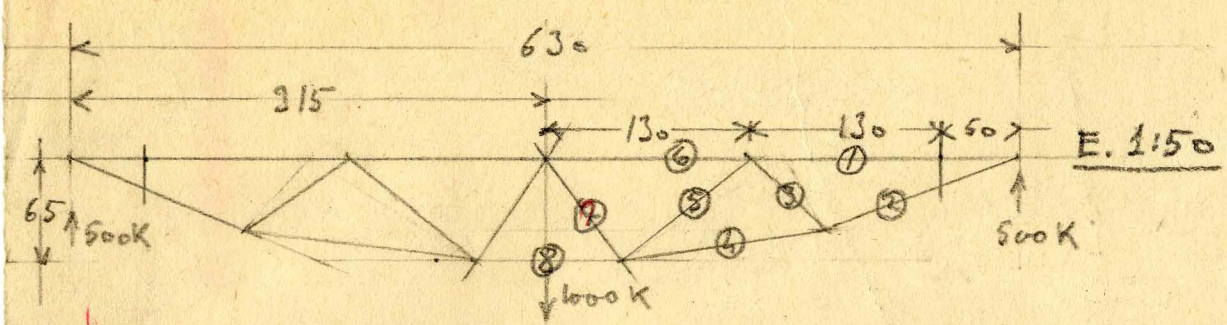
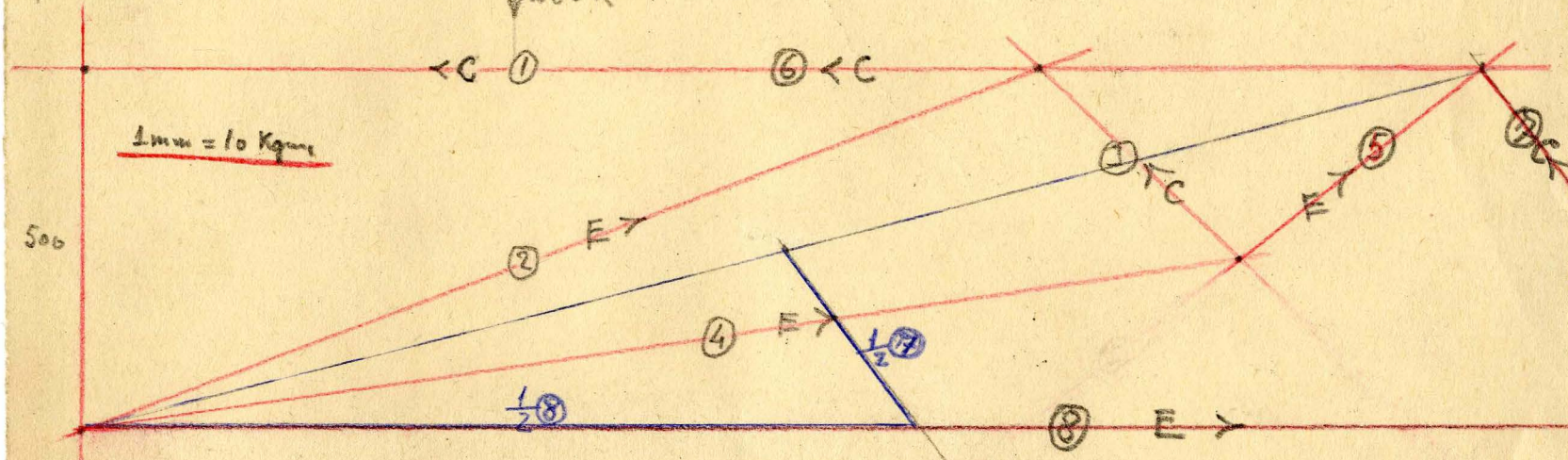
Una Tacena grande armada		270 K	
" " pequeña	"	85 "	
" " mediana	"	105 "	
Cinco cuchillos	armados (a 250)	1250 "	
" columnas	armadas (a 135	675 "	
		2385 K a 0'65	1550'25
mano de obra. electrodos, corriente y jornal de todo este hierro			340
			1890'25

Vesulla el Kº de hierro terminado a 0'79

Peso del cuchillo ejecutado — 1105 K.
2340

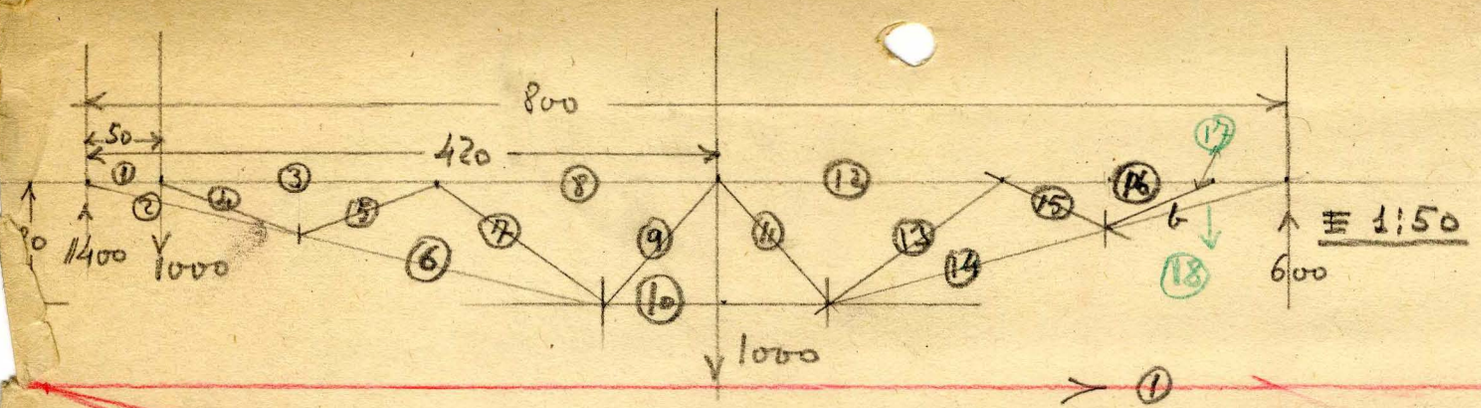




C
- ① = 1340 Kgms.
- ⑥ = 1960 " — $I_{min} \geq 2'5 \times 2 \times \overline{130}^2 = 8'65\ cm^4$ — 2 L de 42x42x6 . I = 14'62 cm⁴
- ③ = 390 Kgms — $I_- \geq 2'5 \times 4 \times \overline{070}^2 = 0'49\ cm^4$
- ⑦ = 620 " — $I_- \geq 2'5 \times 6'7 \times \overline{085}^2 = 0'72\ cm^4$

} — 2 L de 25x25x3 . I = 1'58 cm⁴

E
- ② = 1440 Kgms. — 1'15 cm² } — 2 cm² — 2x5x30 = 2x3 = 3 cm²
- ④ = 1650 " — 1'17 cm² }
- ⑧ = 2340 " — 2'3 cm² — 2'5 cm² — 2x5x40 = 2x2 = 4 cm²
- ⑤ = 430 Kgms. — 0'4 cm⁴ — 2 cm² — 2x5x30 = 2x3 = 3 cm²



Peso del cubillo ejecutado ——— 270 Kgs.

1 mm = 20 Kgms.

Comprimidos

× ① = 6000 Kgs.
✕ ③ = 3300 "
× ⑧ = 2340 "
□ ④ = 2880 "
o ⑦ = 400 "
Σ ⑨ = 540 " 2 de 8×60 →
Σ ⑪ = 800 "
× ⑫ = 2160 "
| ⑮ = 160 "
× ⑯ = 1920 "

Extirados

✱ ② — 7400 Kgs.
| ⑤ — 480 "
✱ ⑥ — 2340 " 2×8×40 →
✱ ⑩ — 2700 "
o ⑬ — 120 "
✱ ⑭ — 2120 "
□ ⑰ — 660 "
✱ ⑱ — 1380 "

2 ⌐ de 65×65×7 " I = 66'8 → 49'50 = ✕ ③ — $I_{min} \geq 2'5 \times 3'3 \times \overline{2'45}^2 =$

2 ⌐ de 35×35×6 . I = 8'28 → 7'25 = □ ④ — $I_{min} \geq 2'5 \times 2'9 \times \overline{1'00}^2 =$

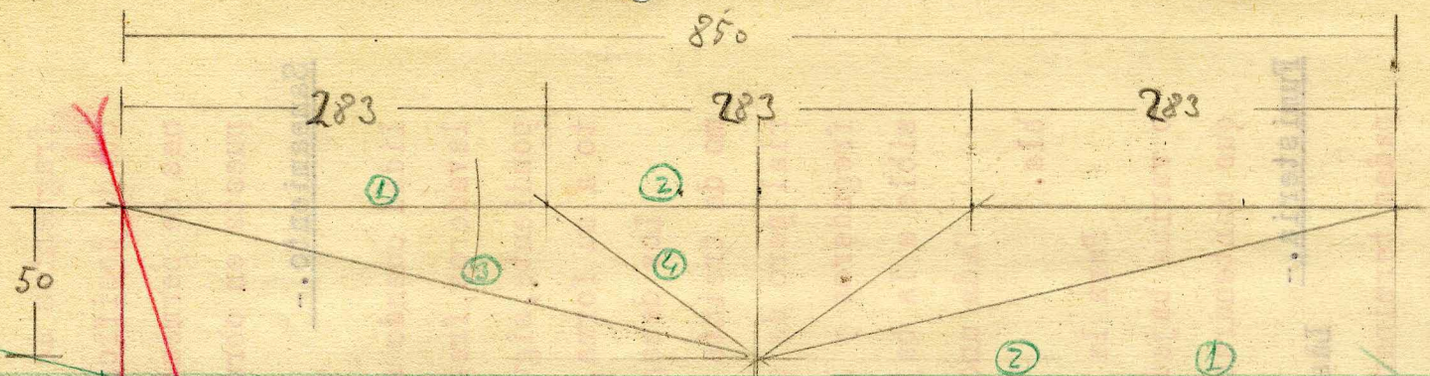
2 ⌐ de 38×38×3 , I = 6'24 → 1'96 = o ⑦ — $I_{min} \geq 2'5 \times 0'4 \times \overline{1'4}^2 =$

2 ⌐ de 38×38×3, I = 6'24 → 2'64 = Σ ⑪ — $I_{min} \geq 2'5 \times 0'8 \times \overline{1'15}^2 =$

2 ⌐ de 24×24×3 , I = 1'44 → 0'50 = | ⑤ — $I_{min} \geq 2'5 \times 0'2 \times \overline{1'00}^2 =$

COMPRIMIDAS

JOAQUIN RIETA VALENCIA



E. 1:50

$M_r = \frac{pl^2}{10} = \frac{178 \times 2'83 \times 283}{10} = \frac{142558}{10}$

Carga por m²	= 50 Kgm.	por m. lineal de viga	50 × 3'70 = 185 Kgm.
" normal "	= 26 "	" " " " "	26 × 3'70 = 97 "
" longitud.	= 48 "	" " " " "	48 × 3'70 = 178 "

Carga en los apoyos 178 × 4'25 = 757 Kgm. ≠ 900 Kgm.
" total = 757 × 2 = 1514 " ≠ 1800 "
" sobre un nudo = 283 × 178 = 503 Kgm. ≠ 600 "
" " tres " = 3 × 503 = 1509 ≠ 1800 "

1 mm = 10 Kgmm.

① = 2570 Kgm.
② = 1174 Kgm.

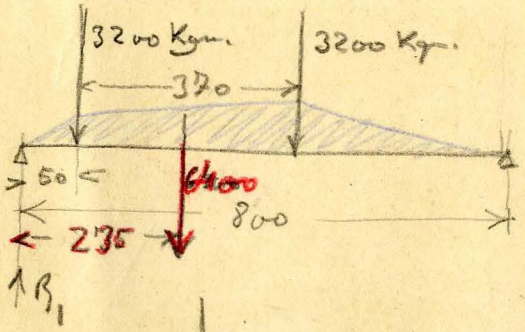
Pmax = 1000 ≷ $\frac{N}{A} \pm \frac{M}{R} = \frac{2570}{A} \pm \frac{14225}{R} = \frac{2570}{2 \times 6'4} \pm \frac{14225}{2 \times 10'8} = 200'2 \pm 658'5 = 859$ Kgm.

[2 ⌐ de 60 × 30 × 6]

③ = 2640 " — dos planos de 2 × 50 × 6 = 600 mm² = 6 cm²

④ = 1300 " Longitud = 2'83 " $I_{mm} \gtrless 2'5 \times 1'3 \times \overline{2'83}^2 = 26$ cm⁴ — dos ⌐⌊ de 50 × 50 × 7 " I = 29'2 cm⁴

Peso del anchillo ejecutado — 250 Kgm.

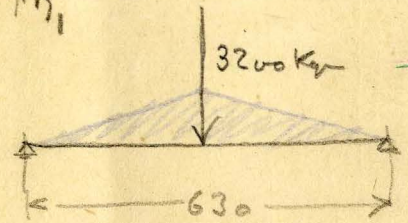


$$R_1 = \frac{6400 \times (8 - 2'35)}{8'00} = \frac{6400 \times 5'65}{8'00} = \frac{36160}{8} = 4520 \text{ Kgr.}$$

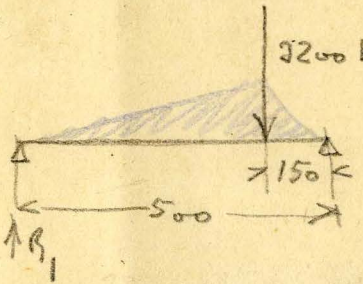
$$R_2 = 6400 - 4520 = 1880 \text{ "}$$

$$\eta_1 = 4520 \times 0'50 = 2260 \text{ Kg.m}$$

$$\eta_2 = 4520 \times 4'20 - 3200 \times 3'70 = 18984 - 11840 = 7144 \text{ Kg.m.}$$



$$\eta_{\rightarrow} = \frac{3200}{2} \times \frac{6'30}{2} = 1600 \times 3'15 = 5040 \text{ Kgr.}$$



$$R_1 = \frac{3200 \times 1'50}{5} = \frac{4800}{5} = 960 \text{ Kgr}$$

$$R_2 = 3200 - 960 = 2240 \text{ Kgr.}$$

$$\eta_{máx} = 960 \times 3'50 = 3360 \text{ Kg. m.}$$

$$\left.\begin{aligned} M &= \Omega \times 1000 \times a \\ \eta &= \Omega \times 1000 \times 1 \\ \eta &= \Omega \times 1000 \times 0'50 \end{aligned}\right\}$$

$$\Omega = \frac{7144}{1000} = 7'14 \text{ m}^2 \quad " \quad \Omega = \frac{7144}{500} = 14'28 \text{ m}^2$$

$$\Omega = \frac{5040}{1000} = 5'040 \text{ m}^2 \quad " \quad \Omega = \frac{5040}{500} = 10'08 \text{ "}$$

$$\Omega = \frac{3360}{1000} = 3'360 \text{ m}^2 \quad " \quad \Omega = \frac{3360}{500} = 6'22 \text{ "}$$