

1 **High strength mortars using ordinary Portland cement-fly ash- fluid catalytic cracking catalyst**
2 **residue ternary system (OPC/FA/FCC)**

3 **L. Soriano¹, J. Payá^{1*}, J. Monzó¹, M.V. Borrachero¹, M.M. Tashima²**

4 ¹ Instituto de Ciencia y Tecnología del Hormigón. Universitat Politècnica de València. Camino de Vera
5 s/n, Edificio 4G, 46022 Valencia, Spain.

6 ² UNESP – Univ Estadual Paulista, Campus de Ilha Solteira. Alameda Bahia, 550. CEP:15385-000 Ilha
7 Solteira-SP, Brazil.

8 *Corresponding author: email: jjpaya@cst.upv.es, telephone: +34 96 3877564.

9 **Abstract**

10 The use of ternary systems composed of ordinary Portland cement (OPC) and two pozzolanic mineral
11 admixtures could supply several advantages in terms of the properties in both fresh and hardened states.
12 Fly ash (FA) and spent fluid catalytic cracking catalyst (FCC) were combined to produce high strength
13 mortars due to a synergic effect. OPC/FA systems (70%/30%) and OPC/FA/FCC systems
14 (70%/20%/10%) were analyzed by thermogravimetric and SEM techniques. Mortars with different
15 binder/sand ratios were prepared in order to yield high compressive strength values. On the one hand, fly
16 ash particles act as nucleation sites that favour the hydration of Portland cement particles: at early stages
17 (7 days), the calculated fixed hydrated lime values were negative, suggestive of a nucleating effect. For a
18 longer curing period (90 days), the pozzolanic effect develops, as can be noted in terms of its compressive
19 strength behaviour. The 90-days curing strength for OPC/FA mortars ranged between 96–98 MPa. In
20 ternary mixtures (OPC/FA/FCC), FCC act as pozzolan during the initial 7 days period; the presence of fly
21 ash particles favoured the presence of more portlandite by means of the nucleation effect. For longer
22 curing times, fly ash particles also contribute to strength development, producing a synergic effect with
23 FCC. The 90 days curing strength for OPC/FA/FCC mortars ranged between 103–106 MPa. Binary and
24 ternary mortars reached strength activity index values equal or higher to the unit. Contributions to the
25 strength (i.e. hydration of cement, the nucleation effect, and early and long term pozzolanic effects) have
26 been calculated for 7 and 90 curing days pozzolan-containing mortars.

27 **Keywords:** pozzolan, fly ash, FCC, high strength mortar, fixed hydrated lime
28
29

30 **1. Introduction**

31 High-strength concrete (HSC) is defined as concrete with high compressive strength [1]. Although there
32 is no precise point of separation between high-strength concrete and normal-strength concrete, the
33 American Concrete Institute defined high-strength concrete as that with a compressive strength greater
34 than 6,000 psi (about 41.37 MPa). Usually, this type of concrete is produced using a low water/binder
35 ratio, a high quantity of Portland cement (OPC) per cubic metre of concrete, and superplasticiser additives
36 and pozzolanic additions such as silica fume or metakaolin. The Spanish code EHE-08 [2] defines high-
37 strength concrete as concrete with a water/binder ratio lower than 0.4 and with a compressive strength
38 higher than 50 MPa (about 7250 psi) for cylindrical specimens (15 x 30 cm).

39
40 As mentioned above, the use of pozzolanic materials in concrete and mortars, especially in HSC, is
41 becoming common practice. Nowadays, the most widely used pozzolanic materials are industrial by-
42 products, and their use in binder composition can contribute to the reduction of OPC consumption,
43 exploitation of raw materials and, consequently, a reduction in the carbon footprint associated with
44 Portland cement-based products. Among the pozzolanic materials, fly ash (FA), metakaolin (MK) and
45 silica fume (SF) are the most used materials [3–8]. The main advantage of using these kinds of material,
46 besides the increment on the mechanical strength, is that they can improve the durability aspects of
47 concrete and mortars such as freeze-thaw resistance, chloride ingress and sulphate attack [9–10].

48
49 The mix proportion for HSC is always associated with high consumption of OPC and high-reactive
50 pozzolan (e.g. silica fume) and water/binder ratios lower than 0.3, a fact that makes the use of a powerful
51 superplasticizer indispensable. The silica fume used for this purpose needs a very small particle diameter
52 in order to promote both the filler and pozzolanic effects, a fact that contributes to the reduction of matrix
53 porosity.

54
55 Due the low water/binder ratio used for HSC compared to that of conventional concrete, the amount of
56 portlandite released during Portland cement hydration is reduced and, consequently, the increment on the
57 compressive strength due the pozzolanic reaction is minimized [11–15].

58

59 Several papers have reported the use of binary and also ternary systems in the production of HSC. Fly ash
60 is the most widely used pozzolanic material due improvements in workability caused by the sphericity of
61 their particles [16–17]. Moreover, fly ash can also improve mechanical strength and durability aspects of
62 concretes over long term curing ages.

63

64 According to multiple studies [18–27], when a low reactivity pozzolan (FA) is used in ternary systems
65 (i.e. OPC plus two mineral admixtures) with another pozzolanic material with high reactivity (SF, MK,
66 etc), a synergic effect between these materials can be observed, it means, the benefits by using both
67 pozzolans are highlighted. The use of FA in ternary systems is justified by its filler effect that acts as a
68 nucleation area for hydrated products produced at early curing ages, increasing the amount of cementing
69 products. Other studies have reported the use of FA in ternary systems for the production of self-
70 compacting concretes. In this case, FA also contributes towards the improvement of fresh concrete
71 workability and the increment of portlandite released due Portland cement hydration to react with high
72 reactive pozzolan [28–31].

73

74 Fluid catalytic cracking catalyst residue (FCC) is a waste material generated in the petrochemical industry
75 and several reports have demonstrated that it has excellent properties as a high reactivity pozzolanic
76 material since the initial days of curing [32–39], presenting an efficiency cementing factor (k-factor)
77 higher than the unit [40]. The main disadvantage of FCC is the reduction in the workability of blended
78 concretes and mortars [32]. Hence, the aim of this paper is to assess the production of high-strength
79 mortars using the ternary system Portland cement/ fly ash/fluid catalytic cracking catalyst residue
80 (OPC/FA/FCC). Both mechanical and microstructural properties will be assessed.

81

82

83 **2. Experimental Program**

84 Portland cement type CEM I 52.5R (OPC), a fly ash class F fly ash (FA, low calcium content) and spent
85 fluid catalytic cracking catalyst (named as FCC) were all used in the production of high strength mortars
86 based on ternary systems. A polycarboxylate based superplasticizer was used in this study. Table 1 shows
87 the chemical composition for all materials used.

88

89 Table 1. Chemical composition for all used materials (wt %).

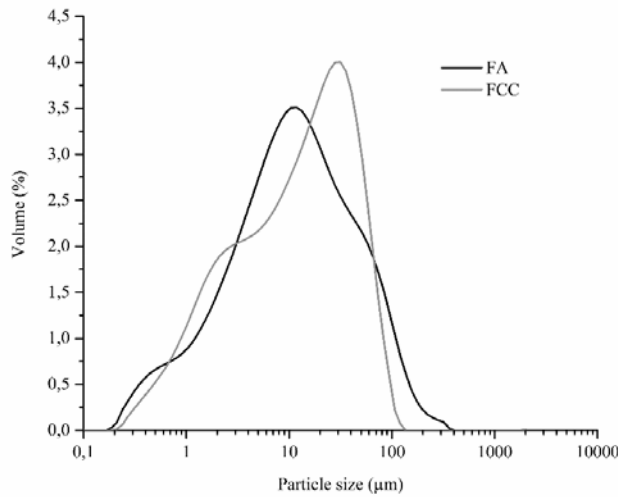
	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	CaO	MgO	SO ₃	K ₂ O	Na ₂ O	L.O.I	Other
FCC	47.76	49.26	0.60	0.11	0.17	0.02	0.02	0.31	0.51	1.24
FA	38.85	24.52	19.63	10.52	1.20	0.47	1.17	0.22	1.56	1.86
OPC	17.42	4.30	3.30	66.17	1.45	3.33	1.21	0.46	2.35	0.01

90
91

92

93 The OPC presented a mean particle diameter of about 20.65 μm, and 50% of particles had a diameter
 94 lower than 44.09 μm. Fly ash (FA) was used as-received from the coal power station, presenting a mean
 95 particle size of about 25.39 μm. Otherwise, FCC was dry milled over 20 minutes in order to reduce its
 96 mean particle size to 19.73 μm, and, consequently, increase its pozzolanic reactivity [28]. Figure 1 shows
 97 the particle size distributions for both FA and FCC mineral admixtures.

98



99

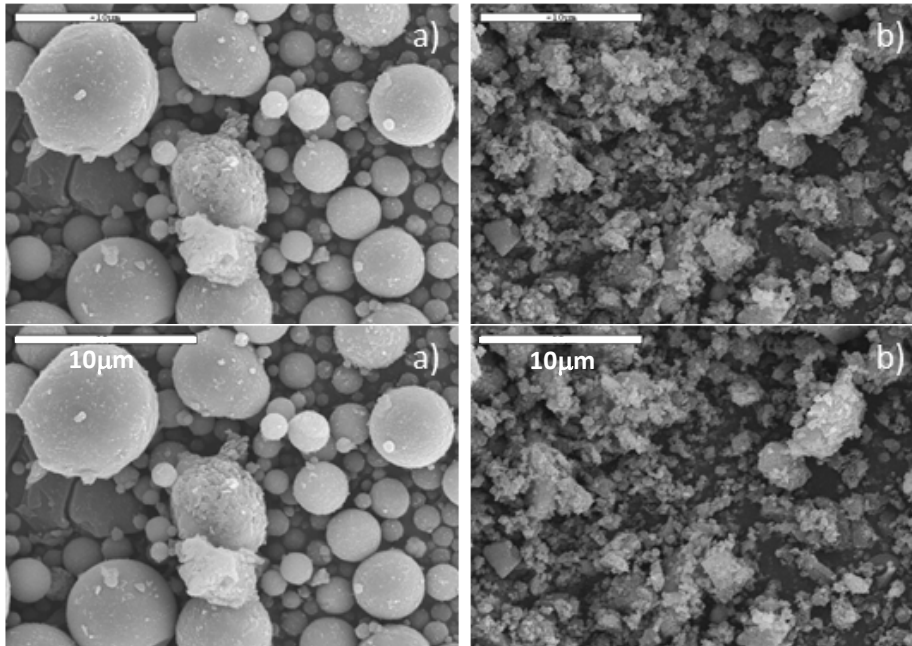
100 Figure 1. Particle size distributions for fly ash (FA) and fluid catalytic cracking catalyst residue (FCC).

101

102

103 The particle morphology of both as-received FA and milled FCC are shown in Figure 2. FA particles
 104 were largely spherical (Fig. 2a), with a wide range of particle diameter. The FCC particles were more
 105 irregular, due to the milling process (Fig. 2b).

106



107

108

109 Figure 2. SEM micrographs: a) as-received FA; b) milled FCC.

110

111

112 The following pastes were prepared: Portland cement paste (this control paste was named as p-CON) and
 113 two OPC/pozzolan pastes: a paste with 70% of OPC and 30% of FA (named p-FA) and a paste
 114 containing 70% of OPC, 20% FA and 10% FCC (named p-FA/FCC). All pastes were prepared using a
 115 water/binder ratio of 0.27 and in order to obtain a homogeneous mixture, and 0.8% (with respect to the
 116 solid mass) of superplasticiser (SP) was added.

117

118 These pastes were placed into cylindrical plastic containers with hermetic closing to avoid any
 119 carbonation process, then they were stored at room temperature with RH ~100% until the testing age (i.e.
 120 7, 28 and 90 curing days).

121

122 In order to stop the hydration process of pastes, paste samples were milled in an agate mortar with
 123 acetone. The milled sample was filtered and the collected solid dried off over one hour at 60 °C. Finally,
 124 the powder sample was sieved through an 80 µm sieve. A Mettler-Toledo TGA 850 equipment was used
 125 to characterise the hydrates formed in the curing process and to assess the calcium hydroxide
 126 consumption by pozzolans in blended pastes. The test was performed from 35 °C to 600 °C, using

127 pinholed aluminium sealed crucibles, with a heating rate of 10 °C/min in a nitrogen atmosphere of 75
128 mL/min.

129

130 From the same pastes, fractured samples were prepared for microstructural analysis. For the scanning
131 electron microscopy (SEM, from JEOL JSM6300), fractured samples were recovered with gold and the
132 analysis was performed using the secondary electron mode.

133

134 Usually, high strength mortars present higher amounts of OPC than conventional mortars. In order to
135 produce this kind of mortar, we decided to modify the reference mortar from the European standard UNE-
136 EN 196-1:2005 [41] (i.e. 450 g of OPC and 1350 g of sand) by adding more OPC. Thus, the cement/sand
137 ratios became: 0.481, 0.569 and 0.667, and the sum of cement and sand was maintained constant for the
138 control mortars. These mortars with selected cement/sand ratios were obtained by replacing 10%, 15%
139 and 20% of sand from the standard mortar (cement/sand ratio of 0.333) by OPC. Thus, the nomenclature
140 for control mortars was: 10%(con), 15%(con) and 20%(con) (see Table 2).

141

142 With respect to the blended mortars produced (Table 2), two classes of binding material were prepared:
143 the first one (binary system OPC/FA) with a proportion of 70% of Portland cement and 30% of FA
144 (named 7-3) ; and the second one (ternary system OPC/FA/FCC) with a proportion of 70% of Portland
145 cement, 20% of FA and 10% of FCC (named 7-2-1: in this case, for three parts of pozzolan, two parts
146 correspond to FA). For all the samples, the water/binder ratio was fixed at 0.27 (the binder being the sum
147 of OPC+FA+FCC). The workability of mortars was performed using a superplasticiser, which was added
148 at a given percentage with respect to the binder (see Table 2). Hence the nomenclature for the prepared
149 mortars is summarized in Table 2: XX(YY), where XX represents the percentage of sand replacement
150 (10%, 15%, 20%) and YY represents the composition of binding material (“con” for only OPC binder;
151 “7-3” for FA binary binder; and “7-2-1” for ternary binder).

152

153 Table 2. Mix proportions of the mortars (SP = superplasticiser).

	cement (g)	FA (g)	FCC (g)	sand (g)	water (g)	SP (%)	Flow Table test (mm)
10 (con)	585.0	-	-	1215.0	157.9	2.5	112.5
10 (7-3)	409.5	175.5	-	1215.0	157.9	2.0	130.0
10 (7-2-1)	409.5	117.0	58.5	1215.0	157.9	2.5	119.5
15 (con)	652.5	-	-	1147.5	176.2	1.8	125.5
15 (7-3)	456.8	195.7	-	1147.5	176.2	1.2	125.5
15 (7-2-1)	456.8	130.5	65.2	1147.5	176.2	2.0	132.5
20 (con)	720.0	-	-	1080.0	194.4	1.3	128.0
20 (7-3)	504.0	216.0	-	1080.0	194.4	0.8	131.0
20 (7-2-1)	504.0	144.0	72.0	1080.0	194.4	1.4	120.5

154

155

156

157

158

159

160 3. Results and discussion

161 3.1. Thermogravimetric studies

162 Three pastes, with water/binder ratios of 0.27, were prepared accordingly to the described in the
 163 experimental section: a control paste (only OPC as binder, p-con), a fly ash containing paste in 30%
 164 replacement of OPC (p-FA), and a ternary paste with 20% of FA and 10% of FCC replacements (p-FA-
 165 FCC). The percentage of fixed calcium hydroxide (%) by pozzolanic reaction was calculated using the
 166 obtained data from the thermogravimetric analysis curves, as proposed by Soriano et al. [36]. Table 3
 167 summarises the fixed calcium hydroxide (%FCH), the total mass loss in the 35-600 °C range (P_T ,%) and
 168 the mass loss corresponding to the dehydroxylation of portlandite (%H_{CH}) in the 520-600 °C range for all
 169 curing ages. In Figure 3, DTG curves for pastes cured at 7, 28 and 90 days are depicted.

170

171 Table 3. Percentage of fixed calcium hydroxide (%FCH), total weight mass loss (% P_T) and mass loss
 172 associated with the dehydroxylation of portlandite (%H_{CH}).

	7 days			28 days			90 days		
	%PT	%HCH	%FCH	%PT	%HCH	%FCH	%PT	%HCH	%FCH
p-con	13.5	1.3	-	15.1	1.4	-	16.3	1.4	-
p-FA	12.6	1.1	-23.5	14.4	1.3	-37.6	15.5	1.3	-32.8
p-FA-FCC	13.2	0.7	22.0	14.8	0.7	29.1	15.3	0.6	32.7

173

174

175 For the control paste, the mass loss associated with the dehydroxylation of portlandite ($\%H_{CH}$) was very
176 low, if compared with values found in previous studies. Thus, Payá et al [34] reported 2.90% for $\%H_{CH}$
177 in 0.5 water/cement ratio at 28 days, and from the results in Soriano et al [36] a value of 3.20% was
178 calculated in similar conditions, while the value of 3.43% was calculated by Pacewska et al [38]. On the
179 other hand, Payá et al [34] reported 1.57% for a paste with a water/cement ratio of 0.25, which was
180 similar to the value reported here (1.39 %) at the same curing age (28 days). This behaviour clearly
181 demonstrates the significant dependence on the available water in cement mixtures in terms of portlandite
182 formation. This fact has a crucial role in the pozzolanic processes when a pozzolan is present in the
183 cementing matrix.

184

185 Pastes with a low water/binder ratio yielded a decreasing of the Portland cement hydration process rate
186 when compared to pastes with high amount of water [34]. Loukili et al. [42] performed a detailed study
187 in high-performance concretes with a water/binder ratio 0.2, and silica fume replacing Portland cement in
188 percentage of 24%, yielding a Portland cement hydration rate for a HSC of about 0.58 for 28 curing days.
189

190 For all curing ages, p-FA pastes produced negative values of fixed calcium hydroxide ($\%FCH$). This fact
191 confirmed that fly ash contributes towards accelerating Portland cement hydration (fly ash particles act as
192 nucleation sites for precipitating hydrates from Portland cement). Similar results were found by Wang
193 [43]. This author studied the effect of fly ash on the hydration process for mixtures with low water/binder
194 ratios and with high percentages of FA (over 25% by volume), concluding that the use of FA accelerates
195 the Portland cement hydration process mainly due the dilution effect caused by the use of pozzolan with
196 reduced mean particle diameters.

197

198 It is important to state that the improvement on the Portland cement hydration (negative values for
199 $\%FCH$) does not mean that the pozzolanic reaction did not develop. Thus, as can be observed in Figure 3,
200 for pastes containing FA, the presence of a defined peak centered in the range 180–240
201 °C was observed. This peak corresponds to the dehydration of calcium aluminate and/or calcium
202 aluminosilicate hydrates (CAH and/or CASH), typical hydrated products formed due the pozzolanic
203 reaction of silicoaluminous pozzolans. This peak was smaller for the control paste, suggesting that the
204 total quantity of CAH/CASH was significantly higher in FA containing paste (p-FA). FA usually reacted

205 at longer curing periods, and this behaviour increased from 28 to 90 days, the fixed lime percentage
206 changed from -37.6% to -32.8%. This reduction in the negative value for %FCH has been attributed to
207 the pozzolanic reactivity of FA, which becomes a more important process than the hydration of Portland
208 cement (developed mainly in the first 28 days).

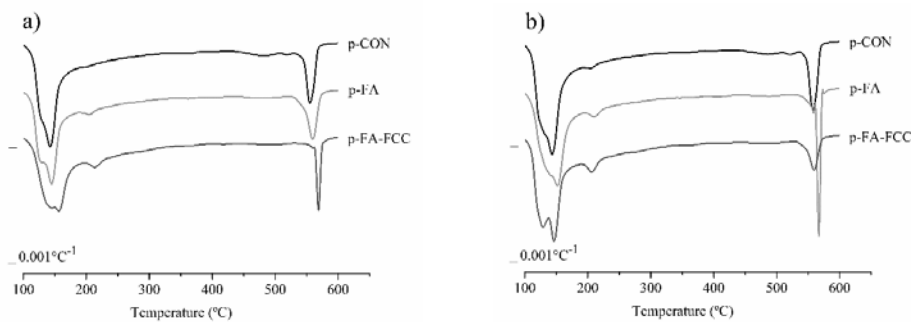
209

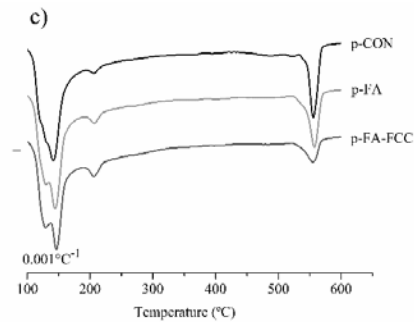
210 Otherwise, when FCC is present in the paste (p-FA-FCC paste), positive values for fixed calcium
211 hydroxide were obtained even for pastes cured during 7 days. For p-FA-FCC paste, the fixed calcium
212 hydroxide was increased for longer curing days. In this case, FCC acts as a high reactivity pozzolanic
213 material, reacting with calcium hydroxide released during Portland cement hydration during the initial
214 curing days. It can be confirmed by the presence of a peak associated with the decomposition/dehydration
215 of CAH and/or CASH (temperature range 180–240 °C): in Figure 3a the peak in this zone was
216 significantly larger than those found in p-con paste and in p-FA paste at 7 days of curing.

217

218 For ternary pastes, the combined effect of FA and FCC promotes a good development of the hydration
219 reactions: FA contributes to the Portland cement hydration, and consequently to the increment on the
220 amount of calcium hydroxide released (portlandite), and FCC reacts with calcium hydroxide in order to
221 form additional products from pozzolanic reaction.

222





223

224 Figure 3. DTG curves for pastes cured at: a) 7 curing days; b) 28 curing days and c) 90 curing days.

225

226 3.2. Scanning electron microscopy studies

227 Fractured paste samples were assessed by scanning electron microscopy (SEM) in order to study the

228 hydration products formed in each paste produced. SEM micrographs for p-FA pastes are shown in

229 Figure 4, and for p-FA-FCC pastes in Figure 5.

230

231 Figures 4a and 4b show an OPC/FA paste cured for 7 days, where the spherical particles of FA can be

232 observed, and they were only slightly recovered by hydration products. Figures 4c and 4d show some FA

233 particles in the cementing matrix that were reacted after 90 days of curing and covered by hydration

234 products: the attack on the FA particles was evident and some of them were fully covered by cementitious

235 products and also were partially dissolved.

236

237 Figures 5a and 5b show that many FA particles were not reacted after the 7 days curing period. Figure 5c

238 shows a typical product formed from FCC pozzolanic reaction (stratlingite), indicating that an early

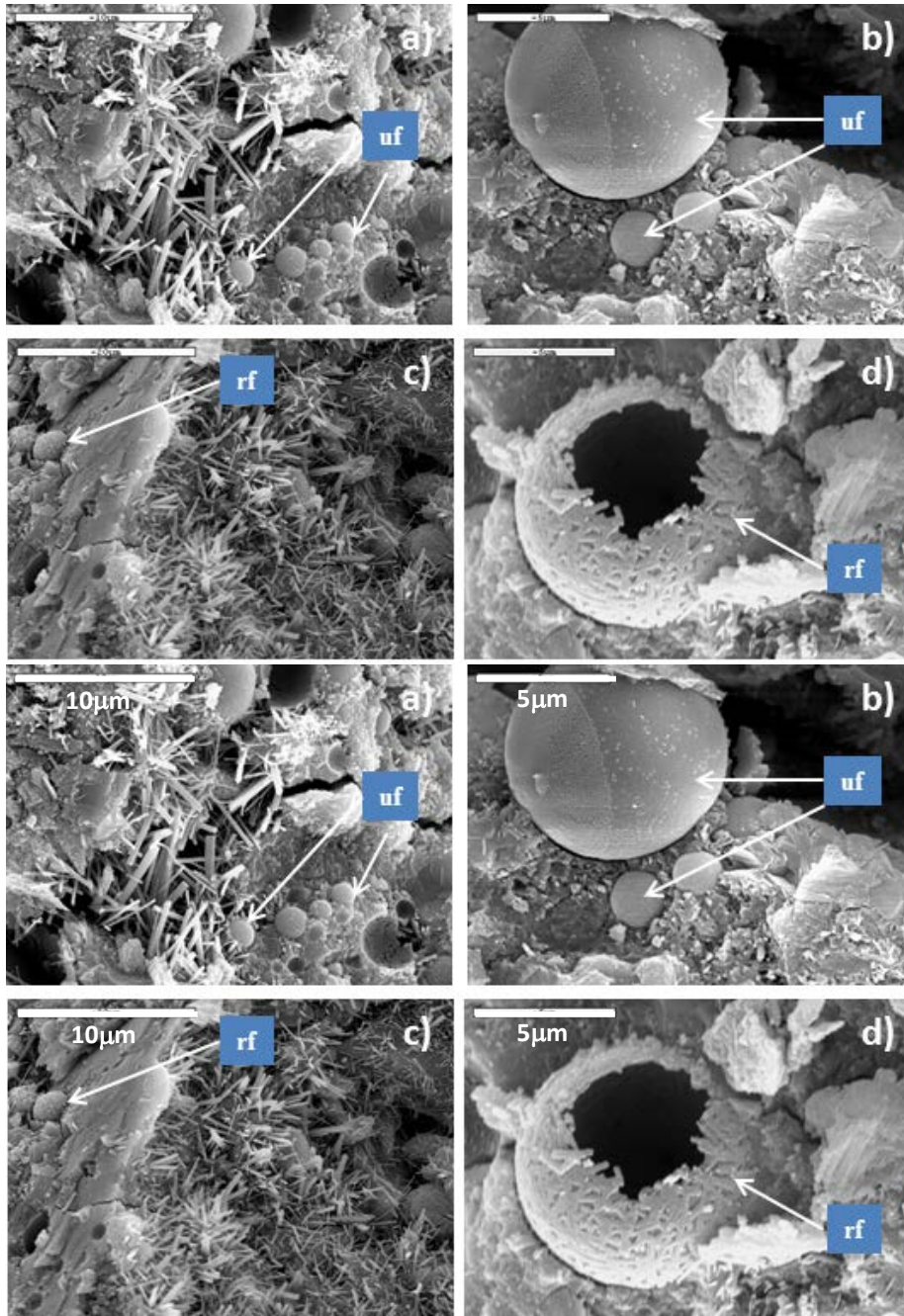
239 pozzolanic reaction took place after 7 days of curing. In Figures 5d-f (90 days curing time), an important

240 group of FA particles was significantly reacted, which indicated that the pozzolanic reaction progress

241 took place for long curing time, despite a significant part of portlandite already having reacted at an early

242 age into FCC particles.

243



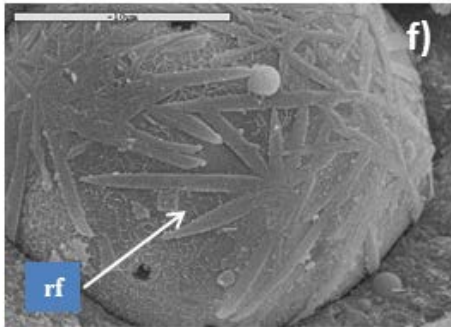
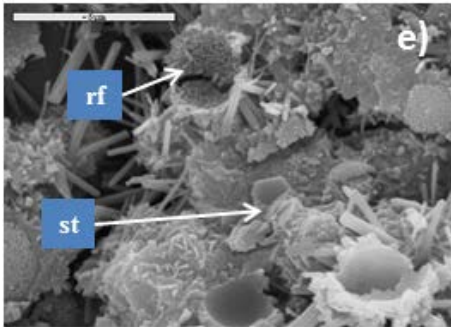
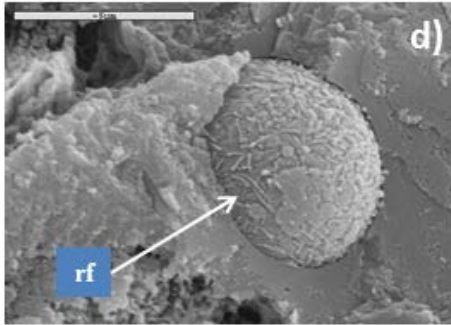
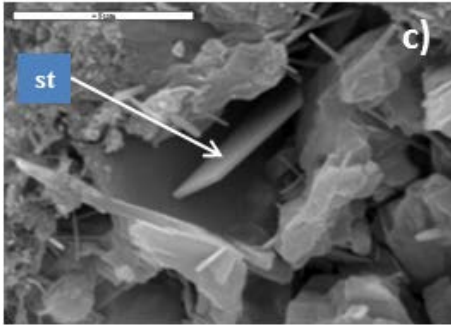
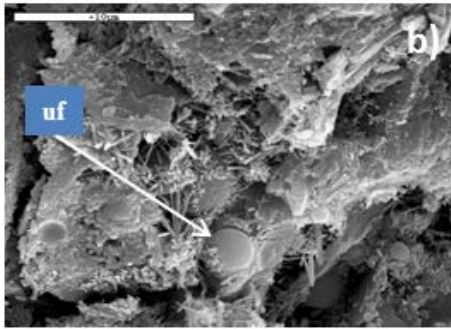
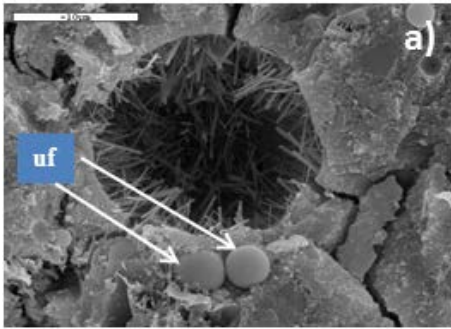
244

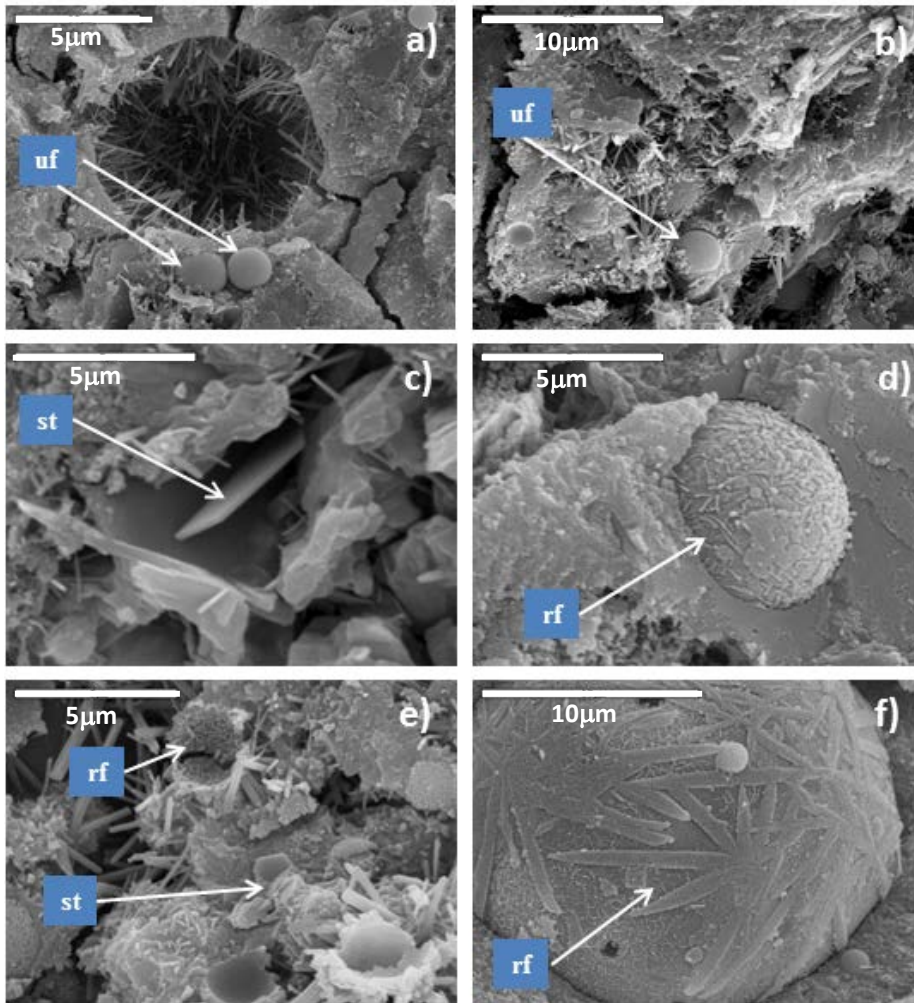
245

246 Figure 4. SEM micrographs for p-7-3 paste (OPC/FA) at different curing ages: a) and b) 7 days; c) and d)

247 90 days. (key: uf: unreacted fly ash; rf: reacted fly ash)

248





250
 251 Figure 5. SEM micrographs for p-7-2-1 paste (OPC/FA/FCC) at different curing ages: a)-c) 7 days; d)-f)
 252 90 days. (key: uf: unreacted fly ash; rf: reacted fly ash; st: stratlingite).

253

254

255 3.3. Mechanical strength studies

256

257 Selected mortars were prepared accordingly to the mix proportions given in Table 2. An increment on the
 258 sand replacement by binder (from 10% to 20%) reduces the required amount of superplasticiser in
 259 mortars for yielding similar workability. This behaviour is due to the increase in the fine particles/sand
 260 ratio.

261

262 Table 4 summarises the flexural (R_f) and compressive (R_c) strengths of high strength mortars tested at 7,
 263 28 and 90 curing days. In general terms, the increase in the binding material content results in a modest
 264 increase in the compressive strength of mortars, over all curing ages. This behaviour was due probably to
 265 the enhancement of the matrix and the interfacial zone by means of the increasing amount of fine
 266 particles. And, as expected, R_c values increased with curing time for all prepared mortars.

268 Table 4: Flexural (R_f , MPa) and compressive (R_c , MPa) strengths of mortars

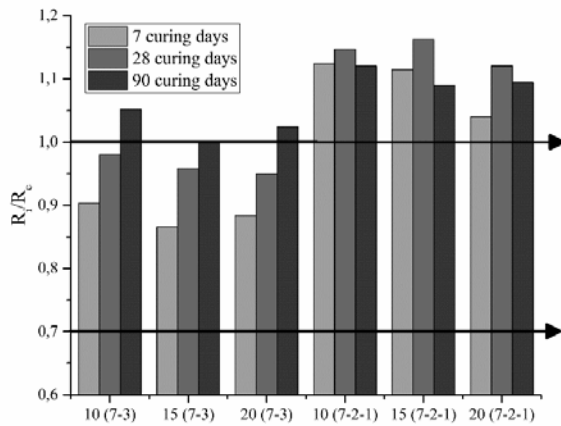
Flexural	7 days	28 days	90 days
10 (con)	12.7±0.6	14.6±0.4	14.9±0.8
10 (7-3)	10.7±0.2	14.1±0.8	13.1±0.6
10 (7-2-1)	13.1±1.0	14.1±0.6	11.9±0.4
15 (con)	14.5±0.8	15.8±0.7	14.5±1.9
15 (7-3)	11.5±0.4	14.9±0.8	13.3±0.8
15 (7-2-1)	13.2±0.7	14.7±0.2	12.1±0.4
20 (con)	14.7±0.3	16.2±1.4	14.3±1.2
20 (7-3)	12.2±1.1	14.7±0.4	14.1±1.0
20 (7-2-1)	13.6±0.6	15.6±0.3	13.5±1.2
Compressive	7 days	28 days	90 days
10 (con)	66.0±0.9	80.9±2.9	92.1±1.3
10 (7-3)	59.6±1.4	79.3±1.8	96.9±3.1
10 (7-2-1)	74.2±3.8	92.8±3.5	103.2±3.5
15 (con)	70.6±3.6	83.7±3.8	96.9±1.1
15 (7-3)	61.1±2.9	80.2±1.1	96.8±2.4
15 (7-2-1)	78.7±2.6	97.3±4.0	105.6±1.0
20 (con)	72.9±3.9	85.4±2.6	95.2±2.8
20 (7-3)	64.4±2.3	81.1±2.1	97.5±3.7
20 (7-2-1)	75.8±4.1	95.7±4.2	104.2±3.6

271
 272 A comparison of control mortars , 10%(con), 15%(con) and 20%(con), with the corresponding mortars
 273 with FA, 10(7-3), 15(7-3) and 20(7-3), shows that for earlier curing ages (7 and 28 days) R_c values for
 274 control mortars were higher than those found for FA mortars. These means that, despite the presence of
 275 fly ash particles that favoured the hydration of Portland cement particles, the strength gain did not
 276 compensate the relative reduction in the content of OPC. However, due to the long-term reactivity for fly

277 ash particles, at 90 days of curing, equal or higher R_c values for FA containing mortars were obtained.
278 This is the typical pozzolanic contribution of fly ash in Portland cement binders. For 90 day curing time,
279 the compressive strength of control mortars ranged from 92.1–96.9 MPa while for FA containing mortars
280 the range was 96.8–97.5 MPa. For the longest curing time studied, the pozzolanic contribution of FA
281 particles plus the increasing in the hydration of OPC particles let to compensate the reduction of 30% of
282 the Portland cement content. A similar trend was found for flexural strength development: thus, at 7 days
283 curing time, the control mortar ranged from 12.7–14.7 MPa, while for FA containing mortar the range
284 was 10.7–12.2 MPa. For the 90 days curing time, strengths increased, reaching values greater than 13
285 MPa for both types of mixtures.

286
287 Blended binders containing FA and FCC, 10(7-2-1), 15(7-2-1) and 20(7-2-1), presented the highest R_c
288 values for all mix proportions and curing times compared to their respective control ones. The high
289 reactivity at early age of FCC came in spite of the reduction of the Portland cement content in the ternary
290 mixtures (30% less than in the control mix), and the low reactivity of fly ash (as can be seen before in the
291 FA containing mixes), the presence of 10% of FCC enhanced significantly the development of strength at
292 7 days. This trend is maintained for 28 days of curing, and 10–13 MPa greater R_c values were found for
293 ternary mortars respect to the control ones. Finally, at 90 days of curing, the contribution of FA in these
294 mortars produced R_c values greater than 100 MPa, significantly higher than those found for the control
295 mortars.

296
297 In order to assess the contribution of pozzolanic materials on the compressive strength of mortars, the
298 strength activity index was determined. This index is defined as the ratio between the strength of the
299 blended mortar and the strength of control mortar [44]. Figure 6 shows the strength activity index for all
300 tested blended mortars.



301

302 Figure 6. Calculated strength activity index for mortars with pozzolanic admixtures.

303

304 The strength activity index for FA blended mortars increased with curing time, yielding the best
 305 behaviour for the mixture 10 (7-3). The obtained data confirms that FA is a pozzolanic material that
 306 reacts for long curing times, i.e. after 28 curing days. However, due to the contribution (nucleation effect)
 307 of FA particles on the hydration of OPC particles, strength activity index values at 7 days curing time fell
 308 in the range of 0.85–0.90, indicating the important role that FA particles played in these systems.

309

310 For FA-FCC blended mortars, for all proportions and for all curing ages the strength activity index was
 311 higher than those obtained for FA containing mortars. The results demonstrated the effectiveness of the
 312 ternary systems both at early and at long term curing ages because strength activity index values were
 313 higher than the unit.

314

315 In order to assess the contribution of both pozzolans in a ternary OPC/FA/FCC system, the following
 316 approach has been developed in terms of compressive strength. The control mortar has a $(R^t)_{OPC}$ strength
 317 at a given curing time “t”. One can consider that the strength is proportional to the relative cement content
 318 (C) with respect to the control mortar as follows:

319

$$320 \quad (R^t)_c = (R^t)_{OPC} * C \quad (1),$$

321

322 where $(R^t)_{OPC}$ is the strength of the control mortar, C is the relative amount of cement in the pozzolan
 323 containing mortar and $(R^t)_c$ is the strength contribution of the cement hydration in pozzolan containing
 324 mortar. Thus, in our case, in which the replacement of OPC was 30%, then C is equal to 0.7, and then
 325 $(R^t)_c$ corresponds to the contribution to the strength due to the relative amount of cement at a given
 326 curing time “t”. $(R^t)_c$ values for 7 and 90 days curing times were calculated and are summarised in Table
 327 5. For 10-con, 20-con and 30-con mortars, $(R^{7d})_c$ were in the range 46.2–51.3 MPa, with a mean value of
 328 44.88 MPa. In the same way, $(R^{90d})_c$ fell in the range 64.47–67.83 MPa, with a mean value of 66.31
 329 MPa.

330 The compressive strength of a fly ash blended cement (OPC/FA) system is calculated as follows:

331

$$332 \quad (R^t)_{FA} = (R^t)_c + (R^t)_{FA,p} \quad (2),$$

333

334 where $(R^t)_{FA}$ is the compressive strength of mortar prepared with the blended cement and $(R^t)_{FA,p}$ is the
 335 pozzolanic contribution to the strength. However, at early ages, fly ash particles do not demonstrate a
 336 pozzolanic reaction, and consequently the second term in Equation (2) would be negligible. Early age
 337 strengths (7 days curing time; see Table 4) for OPC/FA systems were significantly higher than
 338 corresponding $(R^{7d})_c$ values (see Table 5). This behaviour means that there is a nucleation effect and
 339 hydration of Portland cement particles is favored in the presence of FA particles. Thus, Equation (2) must
 340 be transformed into:

341

$$342 \quad (R^{7d})_{FA} = (R^{7d})_c + (R^{7d})_{FA,n} \quad (3),$$

343

344 where $(R^{7d})_{FA,n}$ is the contribution to the strength attributed to the nucleation effect. These values (see
 345 Table 5) ranged from 11.68–13.40 MPa, with a mean value of 12.82 MPa. This contribution enhanced the
 346 production of portlandite, which would be available for pozzolan reaction at longer curing times, or, in
 347 the case of the presence of an additional high reactive pozzolan, would have been reacted at early curing
 348 times.

349 At long curing times (e.g. 90 days), fly ash particles have been partially reacted as showed in SEM
 350 studies, meaning that there is a pozzolanic contribution. Thus, the compressive strength at 90 days for
 351 OPC/FA mortars, $(R^{90d})_{FA}$, is calculated as follows:

352

$$353 \quad (R^{90d})_{FA} = (R^{90d})_c + (R^{7d})_{FA,n} + (R^{90d})_{FA,p} \quad (4),$$

354

355 where $(R^{90d})_{FA,p}$ is the pozzolanic contribution at long term from the fly ash. These calculated values fell
356 in the range of 17.29–19.03 MPa (see Table 5), and the mean value was 19.94 MPa.

357 With respect to the ternary system OPC/FA/FCC, the compressive strength at 7 days would be calculated
358 taking into account that the contribution due to nucleation effect for fly ash is now proportional to the
359 relative pozzolan content (2/3 in our case: for 3 parts of pozzolan, 2 parts correspond to the fly ash).

360 Additionally, the pozzolanic contribution of fly ash would be negligible, $(R^{7d})_{FA,n} \cong 0$, and FCC particles
361 would have a negligible nucleation effect because they are rapidly covered by pozzolanic reaction
362 products according to previous SEM results, $(R^{7d})_{FCC,n} \cong 0$. Thus, the $(R^{7d})_{FCC}$ can be calculated as
363 follows:

364

$$365 \quad (R^{7d})_{FCC} = (R^{7d})_c + \frac{2}{3}(R^{7d})_{FA,n} + (R^{7d})_{FCC,p} \quad (5),$$

366

367 where $(R^{7d})_{FCC,p}$ represents the pozzolanic contribution of FCC in the system. The values related to the
368 pozzolanic contribution of FCC ranged from 15.86–21.49 MPa, with a mean value of 18.81 MPa (see
369 Table 5).

370

371 Finally, the strength for OPC/FA/FCC mixtures at 90 days curing time would be calculated as follows,
372 taking into account the early age pozzolanic contribution from FCC, $(R^{7d})_{FCC,p}$, and the long term
373 pozzolanic contribution from FA, 2/3 the value of the OPC/FA system, $(R^{90d})_{FA,p}$:

374

$$375 \quad (R^{90d})_{FCC,t} = (R^{90d})_c + \frac{2}{3}(R^{7d})_{FA,n} + (R^{7d})_{FCC,p} + \frac{2}{3}(R^{90d})_{FA,p} \quad (6),$$

376

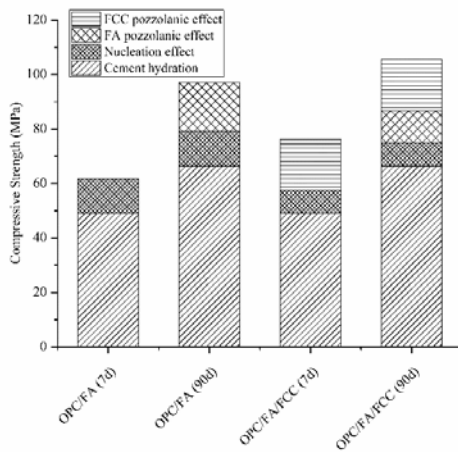
377 where $(R^{90d})_{FCC,t}$ is the theoretical value calculated from Equation (6). Experimental strength values for
378 OPC/FA/FCC system at 90 days curing time, $(R^{90d})_{FCC,e}$, were in the range of 103.2–105.6 MPa (Table
379 4), with a mean value of 104.33 MPa. Theoretical values, $(R^{90d})_{FCC,t}$, were in a similar strength range,

380 103.7–108.64 MPa, with a mean value of 105.62 MPa. The theoretical mean value was very similar to the
 381 experimental one (105.62 MPa vs 104.33 MPa), meaning that our approach proposed is consistent.

382

383 Figure 7 shows the different contributions to the strength for OPC/FA and OPC/FA/FCC systems at 7 and
 384 90 curing days. The pozzolanic contribution of FCC at 7 days of curing was very important due to the
 385 nucleation effect of FA particles, which increased the amount of available portlandite. For the longest
 386 curing time (90 days), the pozzolanic contribution of FA particles was also significant. This provides
 387 evidence of the synergic role of the combination of both pozzolans in the same mixture where each
 388 pozzolanic material presents its pozzolanic contribution but, in the same way, the presence of FA particles
 389 contributes to the enhancement of the nucleation effect thus favoring the pozzolanic reactivity of FCC for
 390 early curing ages.

391



392

393 Figure 7. Compressive strength contributions for OPC/FA and OPC/FA/FCC systems calculated for 7 and
 394 90 days curing times.

395

396 Table 5. Calculated strength (in MPa) terms from Equations (1)–(6).

Mortar dosage	$(R^{7d})_c$	$(R^{90d})_c$	$(R^{7d})_{FA,n}$	$(R^{90d})_{FA,p}$	$(R^{7d})_{FCC,p}$	$(R^{90d})_{FCC,t}$	$(R^{90d})_{FCC,e}$
10%	46.20	64.47	13.40	19.03	19.07	105.16	103.20
15%	49.42	67.83	11.68	17.29	21.49	108.64	105.60
20%	51.03	66.64	13.37	17.49	15.86	103.07	104.20
Mean values	48.88	66.31	12.82	17.94	18.81	105.62	104.33

397

398

399 **4 Conclusions**

400 High-strength mortars were designed using OPC, FA and FCC. Achieving this high strength was carried
401 out by means of low water/binder ratios of 0.27. In these conditions, the following conclusions can be
402 stated in terms of microstructure and mechanical properties:
403 Thermogravimetric data showed that the main role of fly ash particles in an OPC/FA system is the
404 nucleation effect, yielding negative values for the hydrated lime fixation. This effect is related to the
405 excellent strength development at early and long term curing ages in OPC/FA mortars. FA-containing
406 mortars had a pozzolanic contribution for long term ages (i.e. 90 days). SEM studies confirm qualitatively
407 the degree of pozzolanic reaction of FA particles with curing time.
408 At early stages, the nucleation effect of fly ashes produced a higher quantity of portlandite from OPC
409 hydration: this behaviour has a decisive influence on the pozzolanic role of FCC in the OPC/FA/FCC
410 systems. A high percentage of hydrated lime fixation was found, due to the synergic effect at an early age.
411 FCC produced a very important strength contribution, which is attributed to the pozzolanic reaction.
412 Despite to the consumption of portlandite at an early age in the OPC/FA/FCC system, unreacted FA
413 particles also play an important role over longer curing times. In this way, an important contribution to
414 the strength was developed by FA.
415 The combination of both pozzolans led to a synergic effect, which has been elucidated by the production
416 of high-strength mortars with a reduction of 30% of the ordinary Portland cement content.

417
418

419 **Acknowledgements.**

420 This work was supported by Ministerio de Ciencia y Tecnología, Spain (Project MAT 2001-2694).
421 Thanks are given to the Electron Microscopy Service of the Universitat Politècnica de València.

422
423

424 **References**

- 425 [1] Metha PK, Monteiro PJM. Concrete: microstructure, properties and materials. McGraw-Hill, New
426 York, ; 2006.
- 427 [2] EHE-08. Instrucción de hormigón estructural (EHE-08). Real Decreto 1247/2008. B.O.E. nº 203.
428 2008.08.22. Spain

Con formato: Inglés (Estados Unidos)

429 [3] Thomas MDA, Shehata MH, Shashiprakash SG, Hopkins DS, Cail K. Use of ternary cementitious
430 systems containing silica fume and fly ash in concrete. *Cem Concr Res* 1999; 29: 1207–1214. .

431 [4] Erdem TK, Kirca O. Use of binary and ternary blends in high strength concrete. *Constr Build Mater*
432 2008; 22: 1477–1483.

433 [5] Elrahman MA, Hillemeir B. Combined effect of fine fly ash and packing density on the properties of
434 high performance concrete: an experimental approach. *Constr Build Mater* 2014; 58: 225–233.

435 [6] Elahi A, Basheer PAM, Nanukuttan SV, Khan QUZ. Mechanical and durability properties of high
436 performance concretes containing supplementary cementitious materials. *Constr Build Mater* 2010; 24:
437 292–299.

438 [7] Dinakar P, Manu SN. Concrete mix design for high strength self compacting concrete using
439 metakaolin. *Mater Des* 2014; 60: 661–668.

440 [8] Bingöl AF, Tohumcu I. Effects of different regimes on the compressive strength properties of self
441 compacting concrete incorporating fly ash and silica fume. *Mater Des* 2013; 51: 12–18.

442 [9] Peiwei G, Min D, Naiqian F. The influence of superplasticizer and superfine mineral powder on the
443 flexibility, strength and durability of HPC. *Cem Concr Res* 2001; 31: 703–706.

444 [10] Shehata MH, Thomas MDA. Use of ternary blends containing fly ash to suppress expansion due to
445 alkali-silica reaction in concrete. *Cem Concr Res* 2002; 32: 341–349.

446 [11] Ghafari E, Costa H, Júlio E, Portugal A, Durães L. The effect of nanosilica addition on flowability,
447 strength and transport properties of ultra high performance concrete. *Mater Des* 2014; 59: 1–9.

448 [12] Oertel T, Helbig U, Hutter F, Kletti H, SEXTL G. Influence of amorphous silica on the hydration in
449 ultra-high performance concrete. *Cem Concr Res* 2014; 58: 121–130.

450 [13] Oertel T, Hutter F, Helbig U, SEXTL G. Amorphous silica in ultra-high performance concrete: First
451 hour of hydration. *Cem Concr Res* 2014; 58: 131–142.

452 [14] Loukili A, Khelidj A, Richard P. Hydration kinetics, change of relative humidity, and autogenous
453 shrinkage of ultra-high-strength concrete. *Cem Concr Res* 1999; 29: 577–584.

454 [15] Zhao S, Sun W. Nano-mechanical behaviour of a green ultra-high performance concrete. *Constr*
455 *Build Mater* 2014; 63: 150–160.

456 [16] Li Y, Kwan AKH. Ternary blending of cement with fly ash microsphere and condensed silica fume
457 to improve the performance of mortar. *Cem Concr Compos* 2014; 49: 26–35.

- 458 [17] Kwan AKH. Effects of fly ash microsphere on rheology, adhesiveness and strength of mortar. *Constr*
459 *Build Mater* 2013; 42: 137–145.
- 460 [18] Isaia GC, Gastaldino ALG, Moraes R. Physical and pozzolanic action of mineral additions on the
461 mechanical strength of high-performance concrete. *Cem Concr Compos* 2003; 25: 69–76.
- 462 [19] Metha PK, Aitci PC. Principles underlying production of high-performance concrete. *Cem Concr*
463 *Aggr* 1990; 12: 70–80.
- 464 [20] Antiohos S, Maganari K, Tsimas S. Evaluation of blends of high and low calcium fly ashes for use as
465 supplementary cementing materials. *Cem Concr Compos* 2005; 27: 349–356.
- 466 [21] Dehuai W, Zhaoyuan C. On predicting compressive strengths of mortars with ternary blends of
467 cement, GGBFS and fly ash. *Cem Concr Res* 1997; 27: 487–483.
- 468 [22] Wu Z, Naik TR. Properties of concrete produced from multicomponent blended cements. *Cem Concr*
469 *Res* 2002; 32: 1937–1942.
- 470 [23] Bai J, Wild S, Sabir BB. Chloride ingress and strength loss in concrete with different PC-PFA-MK
471 binder compositions exposed to synthetic seawater. *Cem Concr Res* 2003; 33: 353–362.
- 472 [24] Khan MI, Lynsdale CJ, Waldron P. Porosity and strength of PFA/SF/OPC ternary blended paste.
473 *Cem Concr Res* 2000; 30: 1225–1229.
- 474 [25] Barbhuiya SA, Gbagbo JK, Russell MI, Basheer PAM. Properties of fly ash concretes modified with
475 hydrated lime and silica fume. *Constr Build Mater* 2009; 23: 3233–3239.
- 476 [26] Wongkeo W, Thongsanitgarn P, Chaipanich A. Compressive strength and dry shrinkage of fly ash-
477 bottom ash-silica fume multi-blended cement mortars. *Mater Des* 2012; 36: 655–662.
- 478 [27] Wongkeo W, Thongsanitgarn P, Ngamjarurojana A, Chaipanich A. Compressive strength and
479 chloride resistance of self-compacting concrete containing high level fly ash and silica fume. *Mater Des*
480 2014; 64: 261–269.
- 481 [28] Turk K. Viscosity and hardened properties of self-compacting mortars with binary and ternary
482 cementitious blends of fly ash and silica fume. *Constr Build Mater* 2012; 37: 3226–3234.
- 483 [29] Sagmaram M, Christiano HA, Yaman IO. The effect of chemical admixtures and mineral additives
484 on the properties of self-compacting mortars. *Cem Concr Compos* 2006; 28: 432–440.
- 485 [30] Nehdi M, Pardhan M, Koshowski S. Durability of self-consolidating concrete incorporating high-
486 volume replacement composite cements. *Cem Concr Res* 2004; 34: 2103–2112.

487 [31] Güneyisi E, Gesoğlu M, Özbay E. Strength and drying shrinkage properties of self-compacting
488 concretes incorporating multi-system blended mineral admixtures. *Constr Build Mater* 2010; 24: 1878–
489 1887.

490 [32] Payá J, Monzó J, Borrachero MV. Fluid catalytic cracking catalyst residue (FC3R). An excellent
491 mineral by-product for improving early strength development of cement mixtures. *Cem Concr Res* 1999;
492 29: 1773–1779.

493 [33] Payá J, Monzó J, Borrachero MV, Velázquez S. The chemical activation of pozzolanic reaction of
494 fluid catalytic cracking catalyst residue (FC3R) in lime pastes”. *Adv Cem Res* 2007; 19: 9–16.

495 [34] Payá J, Monzó J, Borrachero MV, Velázquez S. Evaluation of the pozzolanic activity of fluid
496 catalytic cracking catalyst residue (FC3R). Thermogravimetric analysis studies on FC3R-Portland cement
497 pastes. *Cem Concr Res* 2003; 33: 603–609.

498 [35] García de Lomas M, Sánchez de Rojas MI, Frías M. Pozzolanic reaction of a spent fluid catalytic
499 cracking catalyst in FCC-cement mortars. *J Therm Anal Calorim* 2007; 90: 443–447.

500 [36] Soriano L, Monzó J, Bonilla M, Tashima MM, Payá J, Borrachero MV. Effect of pozzolans on the
501 hydration process of Portland cement cured at low temperatures. *Cem Concr Compos* 2013; 42: 41–48.

502 [37] Pacewska B, Wilińska I, Bukowska M. Calorimetric investigations of the influence of waste
503 aluminosilicate on the hydration of different cements. *J Therm Anal Calorim* 2009; 97: 61–66.

504 [38] Pacewska B, Wilińska I, Bukowska M, Nocún-Wezelik W. Effect of waste aluminosilicate material
505 on cement hydration and properties of cement mortars. *Cem Concr Res* 2002; 32: 1823–1830.

506 [39] Al-Jabri K, Baawain M, Taha R, Al-Kamyani ZS, Al-Shamsi K. Potential use of FCC spent catalyst
507 as partial replacement of cement or sand in cement mortar. *Constr Build Mater* 2013; 39: 77–81.

508 [40] Payá J, Monzó J, Borrachero MV, Velázquez S. Cement equivalence factor evaluations for fluid
509 cracking catalyst residue. *Cem Concr Compos* 2013; 39: 12–17.

510 [41] UNE-EN 196-1: 2005. Methods of testing cement – part 1: Determination of strength.

511 [42] Loukili A, Khelidj A, Richard P. Hydration kinetics, change of relative humidity, and autogenous
512 shrinkage of ultra-high-strength concrete. *Cem Concr Res* 1999; 29: 577–584.

513 [43] Wang X-Y. Effect of fly ash on properties evolution of cement based materials. *Constr Build Mater*
514 2014; 69: 32–40.

515 [44] Payá J, Borrachero MV, Monzó J, Soriano L. Estudio del comportamiento de diversos residuos de
516 catalizadores de craqueo catalítico (FCC) en cemento Portland. *Mater Constr* 2009; 59: 37–52.